



Illinois Department of Transportation

To: Paul Loete Attn: District Three
From: John D. Baranzelli
Subject: Pavement Design
Date: April 17, 2014

A handwritten signature in black ink, appearing to be 'JDB', enclosed in a hand-drawn oval.

FAP Route 608 (IL 89)
Section (1) BR
Bureau & Putnam Counties
Over the Illinois River at Spring Valley

We have reviewed the pavement design for the above captioned section submitted to BDE on April 4, 2014. The project will reconstruct a portion of IL 89 in conjunction with Spring Valley Bridge over the Illinois River. The project will omit the stabilized sub-base due to a storm sewer system and curb & gutter. The life cycle cost analysis favored the rigid design by more than 10%. The approved pavement design is as follows:

IL 89 over the Illinois River at Spring Valley[new pavement]

9 inches of Jointed PCC Pavement with Tied PCC Curb & Gutter
12 inches of Aggregate Subgrade Improvement

If you have any questions, please contact Paul Niedernhofer at (217) 524-1651.



Illinois Department of Transportation

Memorandum

RECEIVED

APR 07 2014

To: John Baranzelli
From: Paul Loete
Subject: Pavement Design *
Date: April 4, 2014

Attn: Paul Niedernhofer
By: Dave Broviak

BOARD OF
DESIGN & ENVIRONMENT

* FAP 698 (IL 89)
Section (1) BR
Bureau & Putnam Counties
PTB 159/021
Contract No. 66A69
File No. 2100
D3 No. 1366

The attached pavement design is provided for your approval. The project consists of reconstructing 0.93 mile of IL 89. The new bridge is approximately 0.34 mile so the actual new pavement is 0.59 mile. On the north end of the project, the proposed typical section consists of 700' of 26' wide pavement plus B6.24 curb and gutter. The remaining portion of the reconstructed pavement consists of two 12' lanes plus 8' shoulders. The estimated quantity of new pavement is 8,600 square yards plus an additional 4,400 square yards for the shoulders.

The district proposes to use 9" concrete JPC pavement. The 4" stabilized sub-base was not included in the JPC design due to a portion of this roadway having curb and gutter, the proposed use of a 12" Aggregate Subgrade Improvement (ASI) throughout, and the difficulty of constructing a stabilized sub-base throughout multiple stages and entrances.

The following facts and assumptions were used in the design:

- It was assumed "New Construction/Reconstruction" when entering the life cycle cost spreadsheet.
- Design Traffic was based on 2026 projections.
- Design Period of 20 years.
- Poor Sub-grade.
- PG grade 70-22 for top lifts of binder and the surface course.
- The JPC design would not use the optional 4" stabilized sub-base.

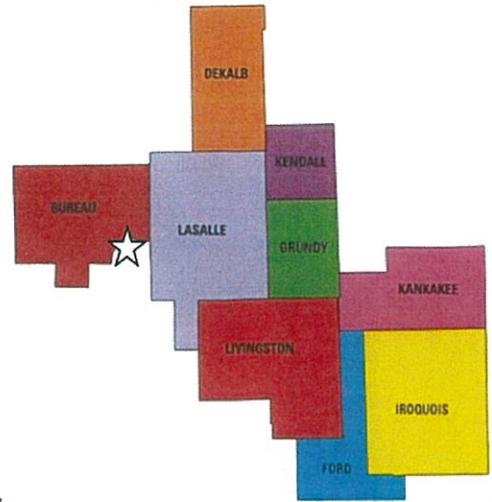
Attachments

Project Location Map

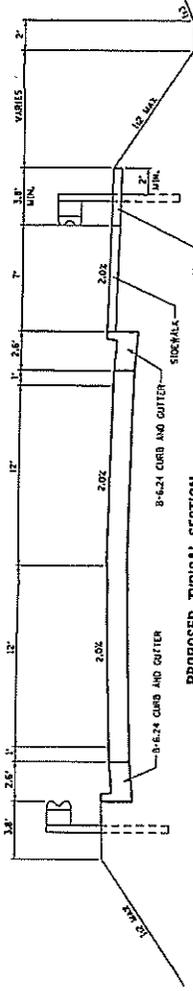
FAP 698 (IL 89)
Section (1) BR
Bureau & Putnam Counties
P-93-013-11 File #2100
D3# 1366 Contract 66A69
SN 078-0006



Project Area = ☆

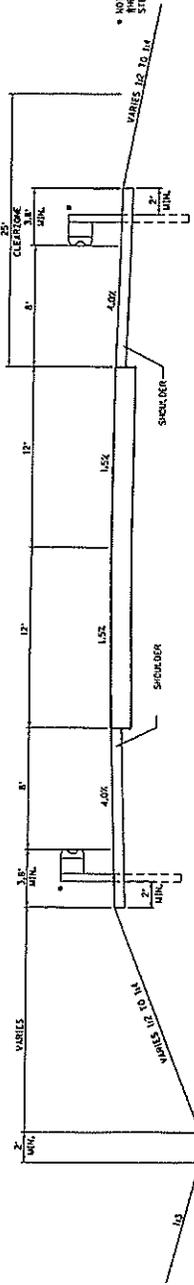


ILLINOIS ROUTE 89 - PROPOSED TYPICAL SECTIONS



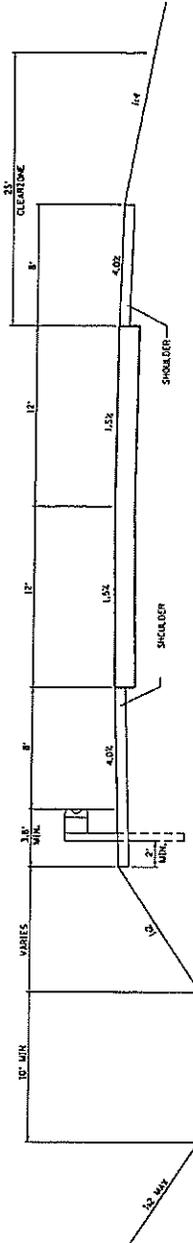
PROPOSED TYPICAL SECTION

SOUTH OF EXISTING RAILROAD BRIDGE TO BARTO BOAT LANDING ENTRANCE, LOOKING SOUTH



PROPOSED TYPICAL SECTION

NORTH OF PROPOSED RIVER BRIDGE, LOOKING SOUTH



PROPOSED TYPICAL SECTION

SOUTH OF PROPOSED RIVER BRIDGE, LOOKING SOUTH

* NOTE: CURB/RAIL IS PROPOSED AT LOCATIONS WHERE THE PROPOSED FRONT SLOPE IS 1:3 OR STEEPER.

PROJECT AND TRAFFIC INPUTS (Enter Data in Gray Shaded Cells)

Route: FAP 689 (IL89)	Comments:	
Section: (1) BR	Design Date: 10/8/2013 DPL	<- BY
County: Bureau & Putnam	Modify Date:	<- BY
Location: Spring Valley		ADT
		Year
		Current: 5,650 2011
		Future: 6,600 2025
Facility Type: Other Marked State Route	# of Lanes = 2 or 3	
	Part of future 4 lanes or more? No	
	One Way Street? No	
	Road Class: II	
	Subgrade Support Rating (SSR): Poor	
	Construction Year: 2016	
	Design Period (DP) = 20 years	

Structural Design Traffic			
	Minimum ADT	Actual ADT	Actual % of Total ADT
PV =	0	6,001	90.0%
SU =	250	333	5.0%
MU =	750	333	5.0%
Struct. Design ADT =	6,668		(2026)

FLEXIBLE PAVEMENT		RIGID PAVEMENT	
Cpv =	0.15	Cpv =	0.15
Csu =	112.06	Csu =	135.78
Cmu =	385.44	Cmu =	567.21
TF flexible (Actual) =	1.67 (Actual ADT)	TF rigid (Actual) =	2.35 (Actual ADT)
TF flexible (Min) =	3.17 (Min ADT Fig. 54-2.C)	TF rigid (Min) =	4.59 (Min ADT Fig. 54-2.C)

TRAFFIC FACTOR CALCULATION

NEW CONSTRUCTION / RECONSTRUCTION PAVEMENT DESIGN CALCULATIONS

Full-Depth HMA Pavement	JPC Pavement
Use TF flexible = 3.17	Use TF rigid = 4.59
PG Grade Lower Binder Lifts = PG 70-22 (Fig. 53-4.R)	Edge Support = Tied Shoulder or C.&G.
HMA Mixture Temp. = 75.5 deg. F (Fig. 54-5.C)	Rigid Pavt Thick. = 9.00 in. (Fig. 54-4.E)
Design HMA Mixture Modulus (E _{HMA}) = 680 ksi (Fig. 54-5.D)	
Design HMA Strain (ε _{HMA}) = 86 (Fig. 54-5.E)	
Full Depth HMA Design Thickness = 10.00 in. (Fig. 54-5.F)	
Limiting Strain Criterion Thickness = 15.00 in. (Fig. 54-5.I)	
Use Full-Depth HMA Thickness = 10.00 inches	

CRC Pavement	
Use TF rigid = 4.59	
IBR value = 3	
CRCP Thickness = 7.75 in. (Fig. 54-4.N)	

TF MUST BE > 60 FOR CRCP

RECONSTRUCTION ONLY (SUPPLEMENTAL) PAVEMENT DESIGN CALCULATIONS

HMA Overlay of Rubblized PCC	Unbonded Concrete Overlay
Use TF flexible = 3.17	Review 54-4.03 for limitations and special considerations.
HMA Overlay Design Thickness = 7.50 in. (Fig. 54-5.U)	
Limiting Strain Criterion Thickness = in. (Fig. 54-5.V)	
Use HMA Overlay Thickness = 999.00 inches	JPCP Thickness = NA inches

CONTACT BMPR FOR ASSISTANCE

DESIGN TABLES FROM BDE MANUAL CHAPTER 54 - PAVEMENT DESIGN

Class I Roads	Class II Roads	Class III Roads	Class IV Roads
4 lanes or more	2 lanes with ADT > 2000	2 Lanes	2 Lanes
Part of a future 4 lanes or more	One way Street with ADT <= 3500	(ADT 750 -2000)	(ADT < 750)
One-way Streets with ADT > 3500			

Facility Type	Min. Str. Design Traffic (Fig 54-2.C)		
	PV	SU	MU
Interstate or Freeway	0	500	1500
Other Marked State Route	0	250	750
Unmarked State Route	No Min	No Min	No Min

Class Table for One-Way Streets	
ADT	Class
0 - 3500	II
>3501	I

Class	Traffic Factor ESAL Coefficients			
	Rigid (Fig. 54-4.C)		Flexible (Fig. 54-5.B)	
	Csu	Cmu	Csu	Cmu
I	143.81	696.42	132.50	482.53
II	135.78	567.21	112.06	385.44
III	129.58	562.47	109.14	384.35
IV	129.58	562.47	109.14	384.35

Class Table for 2 or 3 lanes (not future 4 lane & not one-way street)	
ADT	Class
0 - 749	IV
750 - 2000	III
>2000	II

Number of Lanes	Design Lane Distribution Factors For Structural Design Traffic (Fig. 54-2.B)					
	Rural			Urban		
	P	S	M	P	S	M
1 Lane Ramp	100%	100%	100%	100%	100%	100%
2 or 3	50%	50%	50%	50%	50%	50%
4	32%	45%	45%	32%	45%	45%
6 or more	20%	40%	40%	8%	37%	37%

LIFE-CYCLE COST ANALYSIS: NEW CONSTRUCTION / RECONSTRUCTION

FULL-DEPTH HMA PAVEMENT

Standard Design

ROUTE SECTION COUNTY LOCATION
FAP 689 (IL89)
(1) BR
Bureau & Putnam
Spring Valley

FACILITY TYPE **NON-INTERSTATE**

PROJECT LENGTH **3150 FT ==> 0.60 Miles**
 # OF CENTERLINES **1 CL**
 # OF LANES **2 LANES**
 # OF EDGES **2 EP**
 LANE WIDTH - AVERAGE **12 FT**
 SHOULDER WIDTH HMA Left **8 FT**
 HMA Right **8 FT**
 Total Width of Paved Shoulders **16 FT**

PAVEMENT THICKNESS (FLEXIBLE) **10.00 IN** **15.00 IN MAX**
 SHOULDER THICKNESS **8.00 IN** **HMA_SG Standard Design**
 POLICY OVERLAY THICKNESS **2.25 IN**

FLEX PAVEMENT	TRAFFIC FACTORS	MINIMUM	ACTUAL	USE
		3.17	1.67	3.17

Read Me!

HMA COST PER TON	UNIT PRICE
HMA SURFACE	\$80.15 / TON
HMA TOP BINDER	\$79.21 / TON
HMA LOWER BINDER	\$79.21 / TON
HMA BINDER (LEVELING)	\$83.00 / TON
HMA SHOULDER	\$80.00 / TON

INITIAL COSTS

ITEM	THICKNESS	100% QUANTITY	UNIT	UNIT PRICE	COST
HMA PAVEMENT (FULL-DEPTH)	(10.00")	8,400	SQ YD	\$46.00 / SQ YD	\$386,432 ~
HMA SURFACE COURSE	(2.00")	1,335	947 TONS	\$80.15 / TON	\$0
HMA TOP BINDER COURSE	(2.25")	1,421	1,081 TONS	\$79.21 / TON	\$0
HMA LOWER BINDER COURSE	(5.75")	1,465	2,839 TONS	\$79.21 / TON	\$0
HMA SHOULDER	(8.00")	5,600	2,509 TONS	\$80.00 / TON	\$200,704 ~
CURB & GUTTER		0	LIN FT	\$30.00 / LIN FT	\$0
SUBBASE GRAN MATL TY C (TONS)		0	TONS *	\$25.00 / TON	\$0
IMPROVED SUBGRADE: Modified Soil	Fact: = 0.2	0	SQ YD *	\$7.00 / SQ YD	\$0
Reserved For User Supplied Item		0	UNITS	\$0.00 / UNITS	\$0
Reserved For User Supplied Item		0	UNITS	\$0.00 / UNITS	\$0
PAVEMENT REMOVAL		8,400	SQ YD	\$0.00 / SQ YD	\$0
SHOULDER REMOVAL		5,600	SQ YD	\$0.00 / SQ YD	\$0

Note: * Denotes User Supplied Quantity
 FLEXIBLE CONSTRUCTION INITIAL COST **\$587,136**
 FLEXIBLE CONSTRUCTION ANNUAL COST PER MILE **\$40,139**

MAINTENANCE COSTS:

ITEM	THICKNESS	MATERIAL	UNIT COST
ROUTINE MAINTENANCE ACTIVITY			\$0.00 LANE-MILE / YEAR
HMA OVERLAY PVMT SURF	(2.00")	Surface Mix 2.00	\$9.04 / SQ YD
HMA OVERLAY PVMT	(2.25")	Surface Mix 2.25	\$10.30 / SQ YD
HMA SURFACE MIX	(1.50")	Surface Mix 1.50	\$6.77 / SQ YD
HMA BINDER MIX	(0.75")	Leveling Binder Mix 0.75	\$3.53 / SQ YD
HMA OVERLAY SHLD (Year 30)	(2.25")	Shoulder Mix 2.25	\$10.08 / SQ YD
HMA OVERLAY SHLD	(2.00")	Shoulder Mix 2.00	\$8.96 / SQ YD
MILLING (2.00 IN)			\$3.00 / SQ YD
PARTIAL DEPTH PVMT PATCH (Mill & Fill Surf)		Surface Mix 2.00	\$78.98 / SQ YD
PARTIAL DEPTH SHLD PATCH (Mill & Fill Surf)		Shoulder Mix 2.00	\$78.96 / SQ YD
PARTIAL DEPTH PVMT PATCH (Mill & Fill +2.00")		Leveling Binder Mix 2.00	\$79.30 / SQ YD
PARTIAL DEPTH SHLD PATCH (Mill & Fill +2.00")		Shoulder Mix 2.00	\$78.96 / SQ YD
LONGITUDINAL SHOULDER JOINT ROUT & SEAL			\$1.60 / LIN FT
CENTERLINE JOINT ROUT & SEAL			\$1.85 / LIN FT
RANDOM / THERMAL CRACK ROUT & SEAL (100% Rehab = 110.00' / Station / Lane)			\$2.00 / LIN FT

FLEXIBLE TOTAL LIFE-CYCLE COST **\$864,376**
 FLEXIBLE TOTAL ANNUAL COST PER MILE **\$59,092**

FULL-DEPTH HMA PAVEMENT
HMA OVERLAY OF RUBBLIZED PCC PAVEMENT
Figure 54-7.C
STANDARD DESIGN

MAINTENANCE COSTS:	ITEM	%	QUANTITY	UNIT	UNIT COST	COST	PRESENT WORTH
YEAR 5							
	LONG SHLD JT R&S	100.00%	6,300	LIN FT	\$1.60	\$10,080	
	CNTR LINE JOINT R&S	100.00%	3,150	LIN FT	\$1.85	\$5,828	
	RNDM / THRM CRACK R&S	50.00%	3,465	LIN FT	\$2.00	\$6,930	
	PD PVMT PATCH M&F SURF	0.10%	8	SQ YD	\$78.98	\$632	
	PWFn =	0.8626		PW =	0.8626 X	\$23,470	\$20,245
YEAR 10							
	LONG SHLD JT R&S	100.00%	6,300	LIN FT	\$1.60	\$10,080	
	CNTR LINE JOINT R&S	100.00%	3,150	LIN FT	\$1.85	\$5,828	
	RNDM / THRM CRACK R&S	50.00%	3,465	LIN FT	\$2.00	\$6,930	
	PD PVMT PATCH M&F SURF	0.50%	42	SQ YD	\$78.98	\$3,317	
	PWFn =	0.7441		PW =	0.7441 X	\$26,155	\$19,462
YEAR 15							
	MILL PVMT & SHLD 2.00"	100.00%	14,000	SQ YD	\$3.00	\$42,000	
	PD PVMT PATCH M&F ADD'L 2.00"	1.00%	84	SQ YD	\$79.30	\$6,661	
	HMA OVERLAY PVMT 2.00"	100.00%	8,400	SQ YD	\$9.04	\$75,929	
	HMA OVERLAY SHLD 2.00 "	100.00%	5,600	SQ YD	\$8.96	\$50,176	
	PWFn =	0.6419		PW =	0.6419 X	\$174,766	\$112,176
YEAR 20							
	LONG SHLD JT R&S	100.00%	6,300	LIN FT	\$1.60	\$10,080	
	CNTR LINE JOINT R&S	100.00%	3,150	LIN FT	\$1.85	\$5,828	
	RNDM / THRM CRACK R&S	50.00%	3,465	LIN FT	\$2.00	\$6,930	
	PD PVMT PATCH M&F SURF	0.10%	8	SQ YD	\$78.98	\$632	
	PWFn =	0.5537		PW =	0.5537 X	\$23,470	\$12,995
YEAR 25							
	LONG SHLD JT R&S	100.00%	6,300	LIN FT	\$1.60	\$10,080	
	CNTR LINE JOINT R&S	100.00%	3,150	LIN FT	\$1.85	\$5,828	
	RNDM / THRM CRACK R&S	50.00%	3,465	LIN FT	\$2.00	\$6,930	
	PD PVMT PATCH M&F SURF	0.50%	42	SQ YD	\$78.98	\$3,317	
	PWFn =	0.4776		PW =	0.4776 X	\$26,155	\$12,492
YEAR 30							
	HMA_SD NON-INTERSTATE						
	MILL PVMT & SHLD 2.00"	100.00%	14,000	SQ YD	\$3.00	\$42,000	
	PD PVMT PATCH M&F ADD'L 2.00"	2.00%	168	SQ YD	\$79.30	\$13,322	
	PD SHLD PATCH M&F ADD'L 2.00"	1.00%	56	SQ YD	\$78.96	\$4,422	
	HMA OVERLAY PVMT 2.25 "	100.00%	8,400	SQ YD	\$10.30	\$86,512	
	HMA OVERLAY SHLD 2.25 "	100.00%	5,600	SQ YD	\$10.08	\$56,448	
	PWFn =	0.4120		PW =	0.4120 X	\$202,704	\$83,511
YEAR 35							
	LONG SHLD JT R&S	100.00%	6,300	LIN FT	\$1.60	\$10,080	
	CNTR LINE JOINT R&S	100.00%	3,150	LIN FT	\$1.85	\$5,828	
	RNDM / THRM CRACK R&S	50.00%	3,465	LIN FT	\$2.00	\$6,930	
	PD PVMT PATCH M&F SURF	0.10%	8	SQ YD	\$78.98	\$632	
	PWFn =	0.3554		PW =	0.3554 X	\$23,470	\$8,341
YEAR 40							
	LONG SHLD JT R&S	100.00%	6,300	LIN FT	\$1.60	\$10,080	
	CNTR LINE JOINT R&S	100.00%	3,150	LIN FT	\$1.85	\$5,828	
	RNDM / THRM CRACK R&S	50.00%	3,465	LIN FT	\$2.00	\$6,930	
	PD PVMT PATCH M&F SURF	0.50%	42	SQ YD	\$78.98	\$3,317	
	PWFn =	0.3066		PW =	0.3066 X	\$26,155	\$8,018
							\$277,240
ROUTINE MAINTENANCE ACTIVITY			1.19	Lane Miles	0.00	\$0	\$0
						MAINTENANCE LIFE-CYCLE COST	\$277,240
45	YEAR LIFE CYCLE	CRFn = 0.0407852	MAINTENANCE ANNUAL COST PER MILE				\$18,953

PCC PAVEMENT

JPCP

ROUTE
SECTION
COUNTY
LOCATION

**FAP 689 (IL89)
(1) BR
Bureau & Putnam
Spring Valley**

FACILITY TYPE

NON-INTERSTATE

PROJECT LENGTH **3150 FT ==> 0.60 Miles**
 # OF CENTERLINES **1 CL**
 # OF LANES **2 LANES**
 # OF EDGES **2 EP**
 LANE WIDTH - AVERAGE **12 FT**
 SHOULDER WIDTH **PCC Left 8 FT**
 PCC Right 8 FT
 Total Width of Paved Shoulders **16 FT**

PAVEMENT THICKNESS (RIGID) **JPCP 9.00 IN TIED SHLD**
 SHOULDER THICKNESS **7.50 IN**

POLICY OVERLAY THICKNESS **2.50 IN**

RIGID PAVEMENT	TRAFFIC FACTORS	MINIMUM	ACTUAL	USE
		4.59	2.35	4.59
Worksheet Construction Type is New Construction				The Pavement Type is JPCP

INITIAL COSTS

ITEM	THICKNESS	100% QUANTITY	UNIT	UNIT PRICE	COST
JPC PAVEMENT	(9.00")	8,400	SQ YD	\$39.41 / SQ YD	\$331,044
PAVEMENT REINFORCEMENT		0	SQ YD	\$22.00 / SQ YD	\$0
STABILIZED SUBBASE	(4.00")	9,450	SQ YD	\$0.00 / SQ YD	\$0
PCC SHOULDERS	(7.50" to 7.50")	5,600	SQ YD	\$45.00 / SQ YD	\$252,000
CURB & GUTTER		0	LIN FT	\$30.00 / LIN FT	\$0
SUBBASE GRAN MATL TY C	(- 3.37")	0	TONS *	\$25.00 / TON	\$0
IMPROVED SUBGRADE:	Modified Soil <small>Width = 3.0'</small>	0	SQ YD *	\$7.00 / SQ YD	\$0
Reserved For User Supplied Item		0	UNITS	\$0.00 / UNITS	\$0
Reserved For User Supplied Item		0	UNITS	\$0.00 / UNITS	\$0
PAVEMENT REMOVAL		8,400	SQ YD	\$0.00 / SQ YD	\$0
SHOULDER REMOVAL		5,600	SQ YD	\$0.00 / SQ YD	\$0

Note: * Denotes User Supplied Quantity

RIGID CONSTRUCTION INITIAL COST \$583,044
RIGID CONSTRUCTION ANNUAL COST PER MILE \$39,859

MAINTENANCE COSTS:

ITEM	THICKNESS	MATERIAL	UNIT	UNIT COST
ROUTINE MAINTENANCE ACTIVITY				\$0.00 / LANE-MILE / YEAR
HMA POLICY OVERLAY	(2.50")		2.50	
HMA POLICY OVERLAY PVMT	(2.50")	1.0087	2.50	\$11.48 / SQ YD
HMA SURFACE MIX	(1.50")	1.0062	Surface Mix 1.60	\$6.77 / SQ YD
HMA BINDER MIX	(1.00")	1.0136	elting Binder Mix 1.60	\$4.71 / SQ YD
HMA POLICY OVERLAY SHLD	(2.50")		Shoulder Mix 2.50	\$11.20 / SQ YD
CLASS A PAVEMENT PATCHING				\$180.00 / SQ YD
CLASS B PAVEMENT PATCHING				\$125.00 / SQ YD
CLASS C SHOULDER PATCHING				\$130.00 / SQ YD
PARTIAL DEPTH PVMT PATCH (Mill & Fill HMA Surf)		Surface Mix	1.50	\$76.73 / SQ YD
PARTIAL DEPTH PVMT PATCH (Mill & Fill HMA 2.50")		Surface Mix	2.50	\$81.22 / SQ YD
LONGITUDINAL SHOULDER JOINT ROUT & SEAL				\$1.60 / LIN FT
CENTERLINE JOINT ROUT & SEAL				\$1.85 / LIN FT
REFLECTIVE TRANSVERSE CRACK ROUT & SEAL				\$2.00 / LIN FT
RANDOM CRACK ROUT & SEAL	(100% Rehab = 100.00' / Station / Lane)			\$2.00 / LIN FT

RIGID TOTAL LIFE-CYCLE COST \$734,369
RIGID TOTAL ANNUAL COST PER MILE \$50,204

MAINTENANCE AND REHABILITATION ACTIVITY SCHEDULE

08/12/14

JOINTED PLAIN CONCRETE PAVEMENT
UNBONDED JOINTED PLAIN CONCRETE OVERLAY
Figure 54-7.A

MAINTENANCE COSTS:	ITEM	%	QUANTITY	UNIT	UNIT COST	COST	PRESENT WORTH
YEAR 10							
	PAVEMENT PATCH CLASS B	0.10%	8	SQ YD	\$125.00	\$1,000	
		PWF _n = 0.7441			PW = 0.7441 X	\$1,000	\$744
YEAR 15							
	PAVEMENT PATCH CLASS B	0.20%	17	SQ YD	\$125.00	\$2,125	
		PWF _n = 0.6419			PW = 0.6419 X	\$2,125	\$1,364
YEAR 20							
	PAVEMENT PATCH CLASS B	2.00%	168	SQ YD	\$125.00	\$21,000	
	SHOULDER PATCH CLASS C	0.50%	28	SQ YD	\$130.00	\$3,640	
	LONGITUDINAL SHLD JT R&S	100.00%	6,300	LIN FT	\$1.60	\$10,080	
	CENTERLINE JT R&S	100.00%	3,150	LIN FT	\$1.85	\$5,828	
		PWF _n = 0.5537			PW = 0.5537 X	\$40,548	\$22,450
YEAR 25							
	PAVEMENT PATCH CLASS B	3.00%	252	SQ YD	\$125.00	\$31,500	
	SHOULDER PATCH CLASS C	1.00%	56	SQ YD	\$130.00	\$7,280	
		PWF _n = 0.4776			PW = 0.4776 X	\$38,780	\$18,522
YEAR 30	NON-INTERSTATE						
	PAVEMENT PATCH CLASS B	4.00%	336	SQ YD	\$125.00	\$42,000	
	SHOULDER PATCH CLASS C	1.50%	84	SQ YD	\$130.00	\$10,920	
	HMA POLICY OVERLAY 2.5" (PVMT)	100.00%	8,400	SQ YD	\$11.48	\$96,434	
	HMA POLICY OVERLAY 2.5" (SHLD)	100.00%	5,600	SQ YD	\$11.20	\$62,720	
		PWF _n = 0.4120			PW = 0.4120 X	\$212,074	\$87,372
YEAR 35	NON-INTERSTATE						
	LONGITUDINAL SHLD JT R&S	100.00%	6,300	LIN FT	\$1.60	\$10,080	
	CENTERLINE JT R&S	100.00%	3,150	LIN FT	\$1.85	\$5,828	
	RANDOM CRACK R&S	50.00%	3,150	LIN FT	\$2.00	\$6,300	
	REFLECTIVE TRANSVERSE CRACK R&S	40.00%	2,016	LIN FT	\$2.00	\$4,032	
	PD PVMT PATCH M&F HMA 2.50"	0.10%	8	SQ YD	\$81.22	\$650	
		PWF _n = 0.3554			PW = 0.3554 X	\$26,890	\$9,556
YEAR 40	NON-INTERSTATE						
	PAVEMENT PATCH CLASS B	0.50%	42	SQ YD	\$125.00	\$5,250	
	LONGITUDINAL SHLD JT R&S	100.00%	6,300	LIN FT	\$1.60	\$10,080	
	CENTERLINE JT R&S	100.00%	3,150	LIN FT	\$1.85	\$5,828	
	REFLECTIVE TRANSVERSE CRACK R&S	60.00%	3,024	LIN FT	\$2.00	\$6,048	
	RANDOM CRACK R&S	50.00%	3,150	LIN FT	\$2.00	\$6,300	
	PD PVMT PATCH M&F HMA 2.50"	0.50%	42	SQ YD	\$81.22	\$3,411	
		PWF _n = 0.3066			PW = 0.3066 X	\$36,917	\$11,317
							\$151,325
	ROUTINE MAINTENANCE ACTIVITY		1.19	Lane Miles	\$0.00	\$0	\$0
							MAINTENANCE LIFE-CYCLE COST \$151,325
45	YEAR LIFE CYCLE	CRF _n = 0.0407852					MAINTENANCE ANNUAL COST PER MILE \$10,345

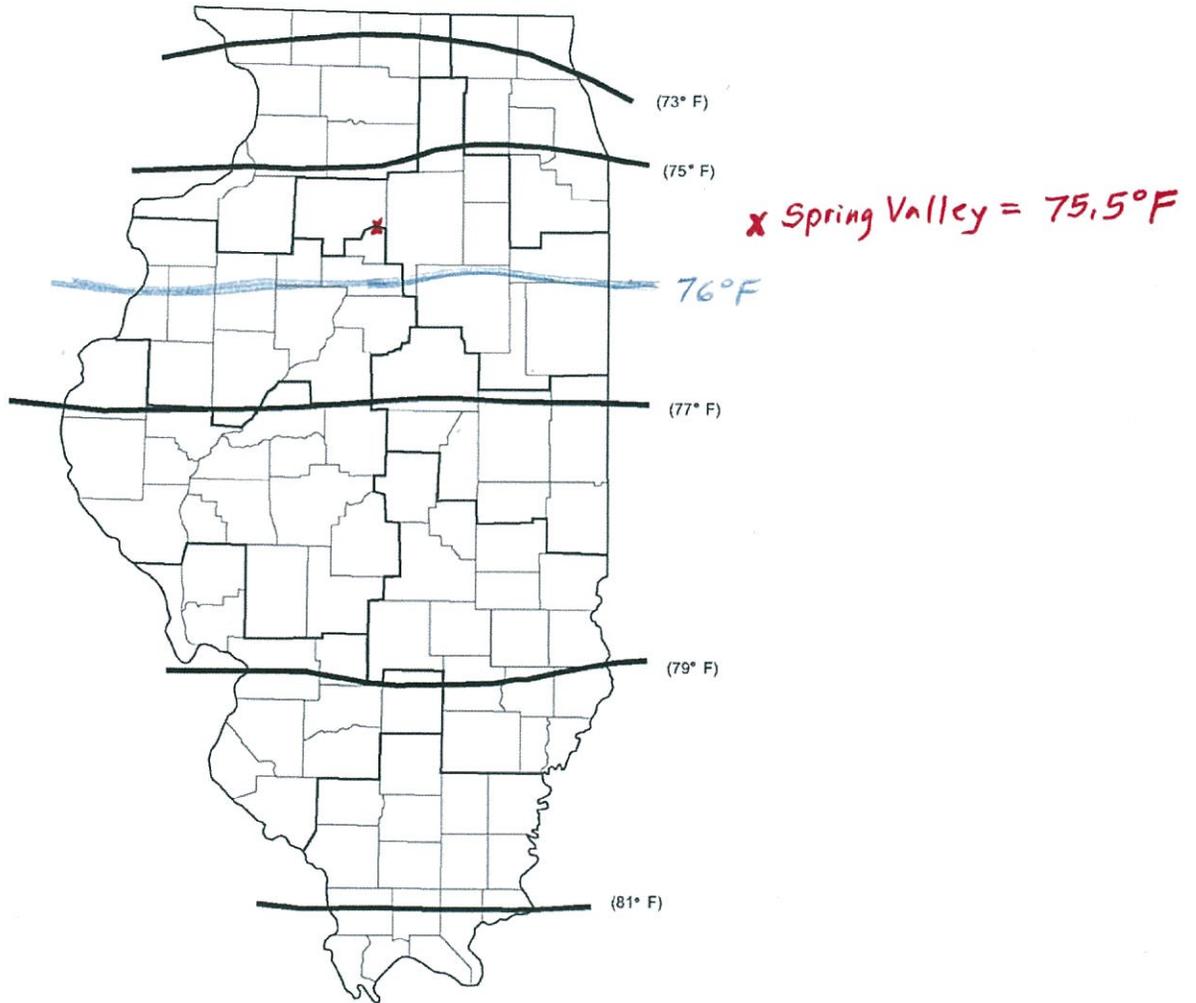
LIFE-CYCLE COST ANALYSIS: NEW DESIGN

Calculated / Revised : 2/28/14 9:57 AM

			JPCP	HMA
CONSTRUCTION	INITIAL COST	PRESENT WORTH	\$583,044	\$587,136
		ANNUAL COST PER MILE	\$39,859	\$40,139
MAINTENANCE	LIFE-CYCLE COST	PRESENT WORTH	\$151,325	\$277,240
		ANNUAL COST PER MILE	\$10,345	\$18,953
TOTAL	LIFE-CYCLE COST	PRESENT WORTH	\$734,369	\$864,376
		ANNUAL COST PER MILE	\$50,204	\$59,092

LIFE-CYCLE COST ANALYSIS: FINAL SUMMARY

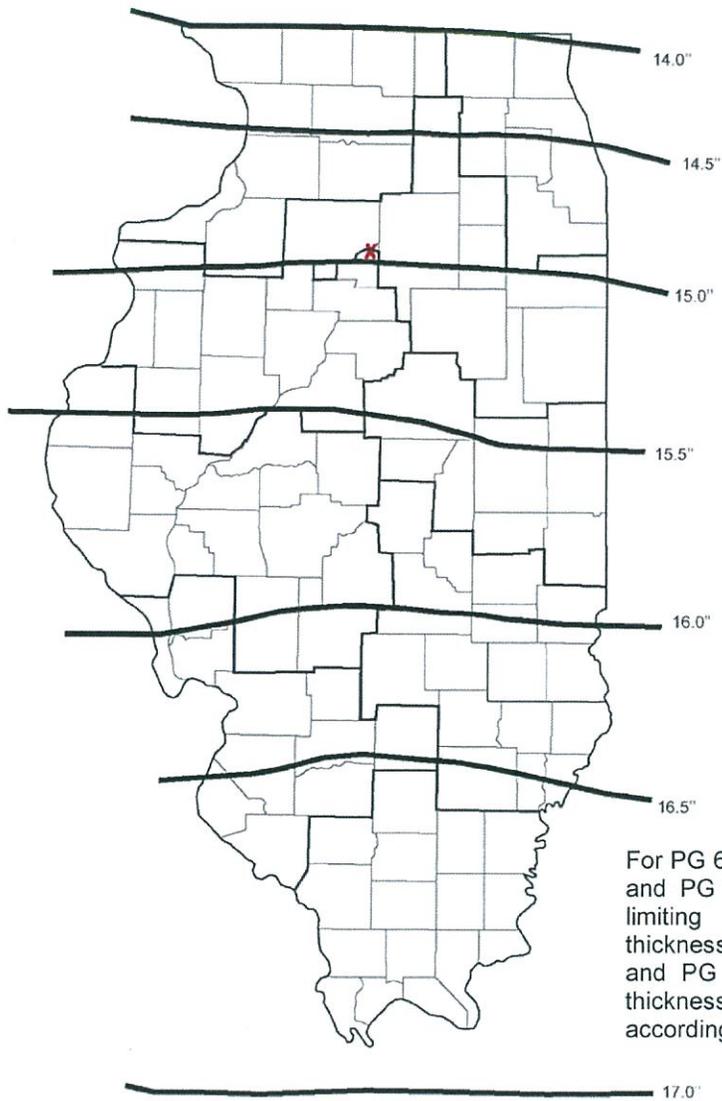
LOWEST COST OPTION	=====>	JPCP	\$50,204	
OTHER OPTIONS (LOWEST TO HIGHEST):	TYPE / PERCENTAGE	HMA	\$59,092	17.7%



Note: The minimum design HMA mixture temperature will be 73°F.

HMA MIXTURE TEMPERATURE
(Mechanistic Design: Flexible Pavement)

Figure 54-5.C



= 15"

Lower lift = PG 70-22
(Do not add 1")

Select lower value
from Fig's. 54-5.F 10 1/4"
and this one 15"

For PG 64-22, PG 70-22, PG 76-22,
and PG 76-28 binder grades, read
limiting strain criterion design
thickness off of map. For PG 64-28
and PG 70-28 binder grades, the
thickness should be corrected
according to Section 54-5.01(i)8a.

Note. Thickness values based upon Mean Monthly Pavement Temperature at 4 in. depth correlated to July Mean Monthly Air Temperature, axle load of 20,000 lb, strain of 70 $\mu\epsilon$, and E_{Ri} of 2 ksi.

MAXIMUM PAVEMENT THICKNESS
(Limiting Strain Criterion Design: Flexible Pavement)

Figure 54-5.1