

IDOT - Division of Highways - District One
September 2017

## ENGINEERING DESIGN

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## Appendix A-1

## Design Criteria Checklist

## Level Two Design Criteria Checklist

| Key Route: $\quad$ F.A.P. Route 47 |  |  |
| :---: | :---: | :---: |
| Marked Route/Road Name: Illinois Route 47 |  |  |
| P-91-101-07 | Contract No.: |  |
| Functional Classification: Strategic Regional Arterial | Highway Type: | Rural/Suburban Arterial |
| McHenry | Project Length: | 7.6 miles ( 40,425 feet) |
| Dorr and Grafton townships | Section: |  |
| Project Location: IL Route 47-Reed Road to U.S. 14 |  |  |

## Project Scope of Work

a. Check the appropriate box. See Section 31-6 for definitions.

| $\square$ | New construction | $\boxtimes$ | *Reconstruction | $\square$ | *3R (non-freeway) | $\square$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\square$ | 3P | $\square$ | SMART (freeway) |  |  |  |
| $\square$ | $\square$ | HSIP | $\square$ | Other |  |  |

*Note: May include "Allowed to Remain in Place" criteria.
This form is required for all new construction, reconstruction, and $3 R$ projects.
b. Provide a brief project description:

Reconstruction of 7.6 miles with two lanes in each direction separated by a raised-curb median in the suburban section and wide depressed median in rural areas. Accommodations for an 8 -feet multi-use path and 5 feet sidewalk are also provided throughout the project limits.

In the suburban area starting at Reed Road to Rainsford Drive, there is a 22 feet raised curb median with 10 feet outside shoulders.

In the rural section from Rainsford Drive to Hercules Road, there is a 30 feet wide depressed median with 6 feet inside shoulders. The two lanes in each direction slopes $1 / 4$ "/ft ( $2.0 \%$ ) at the center of the two lanes with 10 feet outside shoulders and a M-4.24 curb.

In the suburban section between Hercules Road to US 14, the pavement cross slope is $1 / 4^{\prime \prime} / \mathrm{ft}$ sloping away from the 18 feet raised curb ( $B-6.12$ ) median with $\mathrm{B}-6.24$ combination concrete curb and gutter.

| Design Criteria <br> （Provide numerical values，where indicated．） | Does the proposed design meet the criteria？ |  |  |
| :---: | :---: | :---: | :---: |
|  | Yes | No | N／A |
| 1．Basic Design Controls（Chapter 31） |  |  |  |
| a．Design 50 （Reed to Rainsford－Suburban）mph speed 60 （Rainsford to Hercules－Rural）（km／h） 45 （Hercules to US 14－Suburban） | 区 | $\square$ | $\square$ |
| b．Stopping Sight Distance（SSD）application for vertical curves（downgrade adjusted SSD used） | 囚 | $\square$ | $\square$ |
| c．Truck SSD（level）（at specific sites） | $\square$ | $\square$ | 囚 |
| d．Level of service（mainline） <br> The design projects the LOS to be B in 2040. | 区 | $\square$ | $\square$ |
| 2．Horizontal Alignment（mainline）（Chapter 32） |  |  |  |
| a．Horizontal curvature（minimum radius for selected design speed） 1330 feet（Rural）and 715 feet （Suburban）feet（meters） | 区 | $\square$ | $\square$ |
| b．Superelevation rates（ $\mathrm{e}_{\text {max }}=6 \%$ ） | 区 | $\square$ | $\square$ |
| c．Superelevation transition lengths <br> Varies，Tangent Runout＋SE Runoff Length | 区 | $\square$ | $\square$ |
| d．SSD application at horizontal curves（downgrade adjusted SSD used） | 区 | $\square$ | $\square$ |
| e．Superelevation distribution between tangent and curve（ratio or percent） $\begin{aligned} & 67 \% \text { tangent and } \\ & 33 \% \text { curve }\end{aligned}$ | 区 | $\square$ | $\square$ |
| f．＂Breakover＂of outside shoulder on super－ elevated curves（percent）8\％ | 区 | $\square$ | $\square$ |
| g．Relative longitudinal slope of shoulder to edge of traveled way on high side of S．E．curve adjacent to bridge with S．E． | $\square$ | $\square$ | 区 |
| h．Superelevation development at reverse curves | $\square$ | $\square$ | 囚 |


| Design Criteria （Provide numerical values，where indicated．） | Does the proposed design meet the criteria？ |  |  |
| :---: | :---: | :---: | :---: |
|  | Yes | No | N／A |
| i．Is superelevation transition length located off of bridges and bridge approach pavements？ <br> There is no superelevation near the Kishwaukee River crossing． | 区 | $\square$ | $\square$ |
| j．Horizontal stopping sight distance on inside of horizontal curves（Level SSD for passenger cars） | 区 | $\square$ | $\square$ |
| 3．Vertical Alignment（mainline）（Chapter 33） |  |  |  |
| a．Maximum grades（in percent） $3.96 \%$（Rural）and $1.34 \%$（Suburban） | 区 | $\square$ | $\square$ |
| b．SSD at crest vertical curves（level SSD for passenger cars） | 区 | $\square$ | $\square$ |
| c．SSD at sag vertical curves（level SSD for passenger cars） | 区 | $\square$ | $\square$ |
| d．Minimum grades（in percent）considering drainage 0．50\％（Rural）and 0．40\％（Suburban） | 区 | $\square$ | $\square$ |
| e．Critical length of grade <br> Design per BDE Figure 33－2A | 区 | $\square$ | $\square$ |
| f．Truck－climbing lanes／critical grade analysis | $\square$ | $\square$ | 区 |
| g．Design criteria for truck－climbing lanes（e．g．，lane width and shoulder width） | $\square$ | $\square$ | 区 |
| h．Minimum length of vertical curves for selected design speed <br> 3 V ，where V is the design speed in $\mathrm{mph}(\mathrm{km} / \mathrm{hr}$ ） | 区 | $\square$ | $\square$ |
| i．Maximum length of vertical curves（drainage of curbed facilities and bridges） 760 feet | 区 | $\square$ | $\square$ |
| 4．Cross Section Elements（mainline）（Chapter 34） |  |  |  |
| a．Lane widths 12 feet（meters） | 区 | $\square$ | $\square$ |


| Design Criteria <br> （Provide numerical values，where indicated．） | Does the proposed design meet the criteria？ |  |  |
| :---: | :---: | :---: | :---: |
|  | Yes | No | N／A |
| b．Traveled way widening | 区 | $\square$ | $\square$ |
| c．Cross－slopes on through lanes（in percent）： | $\square$ $\square$ $\square$ $\square$ | 区 $\square$ $\square$ $\square$ | $\square$ $\square$ 区 区 |
| d．Shoulder widths $\qquad$ feet（meters）（inside） feet（meters）（outside） | $\begin{aligned} & \text { 区 } \\ & \boxtimes \end{aligned}$ | $\square$ | $\square$ |
| e．Design of parking lanes： <br> －Cross－slope $\qquad$ \％ <br> －Width $\qquad$ feet（meters） |  |  | $\begin{aligned} & \boxtimes \\ & \boxtimes \end{aligned}$ |
| f．Type of curb and gutter used on median B-6.24 | 区 | $\square$ | $\square$ |
| g．Drainage of raised curb medians： <br> －Direction of flow of median surface or pavement Towards Gutter <br> －Direction of cross－slope on gutter $\qquad$ <br> 6 $\%$ | $\begin{aligned} & \boxtimes \\ & \boxtimes \end{aligned}$ | $\begin{aligned} & \square \\ & \square \end{aligned}$ | $\begin{aligned} & \square \\ & \square \end{aligned}$ |
| h．Type of curb and gutter used along outside edges of pavement $\qquad$ M－4．24 | 区 | $\square$ | $\square$ |
| i．Two Way Left Turn Lane（TWLTL）width： <br> －Flush type $\qquad$ feet（meters） <br> －Traversable type $\qquad$ feet（meters） | $\begin{aligned} & \square \\ & \square \end{aligned}$ | $\begin{aligned} & \square \\ & \square \end{aligned}$ | $\begin{aligned} & \boxtimes \\ & \boxtimes \end{aligned}$ |
| j．Median widths： <br> －Urban feet（meters） <br> －Suburban $\qquad$ <br> －Rural $\qquad$ feet（meters） feet（meters） | $\begin{aligned} & \square \\ & \boxtimes \end{aligned}$ | $\square$ $\square$ $\square$ | 区 $\square$ $\square$ |
| k．Shoulder cross slopes 4 \％ | 区 | $\square$ | $\square$ |
| I．Fill slopes $3: 1$ | 区 | $\square$ | $\square$ |


| Design Criteria <br> （Provide numerical values，where indicated．） | Does the proposed design meet the criteria？ |  |  |
| :---: | :---: | :---: | :---: |
|  | Yes | No | N／A |
| m．Outside roadway ditch： | 区 | $\begin{aligned} & \square \\ & \boxtimes \end{aligned}$ | $\begin{aligned} & \square \\ & \square \end{aligned}$ |
| －Slopes 3：1－Depth Varies |  |  |  |
| －Widths 4 feet |  |  |  |
| －Slopes 2 feet－Depth 4：1 | $\square$ | 区 | $\begin{aligned} & \square \\ & \square \end{aligned}$ |
| －Width Varies |  |  |  |
| n．Cross－section transitions into bridges／ underpasses | $\square$ | $\square$ | 区 |
| o．Use of mountable curbs（ $\mathrm{V}>45 \mathrm{mph}(70 \mathrm{~km} / \mathrm{h})$ ） M－4．24 | 区 | $\square$ | $\square$ |
| p．Cross－section transition details（e．g．，four－lane to two－lane） | $\square$ | $\square$ | 区 |
| 5．Intersections（Chapter 36） |  |  |  |
| a．Accommodation of design vehicle （identify vehicle） <br> WB－65 | 区 | $\square$ | $\square$ |
| b．Level of service： <br> －Through lanes <br> －Turn lanes $\qquad$ $\frac{C}{D}$ | $\begin{aligned} & \boxtimes \\ & \boxtimes \end{aligned}$ | $\begin{aligned} & \square \\ & \square \end{aligned}$ | $\begin{aligned} & \square \\ & \square \end{aligned}$ |
| c．Skew angle <br> The angle is more than 15 degrees at Lucas Road， but less than 30 degrees | 区 | $\square$ | $\square$ |
| d．Profiles <br> All profiles meet design criteria． | 区 | $\square$ | $\square$ |
| e．Volume guidelines for turn－lanes： <br> －Right－turns <br> －Left turns | $\begin{aligned} & \boxtimes \\ & \boxtimes \end{aligned}$ | $\begin{aligned} & \square \\ & \square \end{aligned}$ |  |
| f．Design of right－turn lanes Design of left－turn lanes |  | $\begin{aligned} & \square \\ & \square \end{aligned}$ | $\square$ $\square$ |



| 6. Interchanges (Chapter 37) |  |  |  |  |
| :--- | :--- | :---: | :---: | :---: |
| a. Exit <br> terminal | Standard type <br> Design speed of first curve <br> Are any exit terminals located <br> on mainline horizontal curve? | $\square$ | $\square$ | $\boxtimes$ |


| Design Criteria <br> （Provide numerical values，where indicated．） |  |  |  | Does the proposed design meet the criteria？ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Yes | No | N／A |
| b．Entrance terminal | Standard type |  |  | $\square$ | $\square$ | 区 |
|  | Length of tangent after the entering curve |  |  | $\square$ | $\square$ | 区 |
|  | Design speed of entering curve |  |  | $\square$ | $\square$ | 区 |
| c．Design speed of ramp proper |  |  |  | $\square$ | $\square$ | 区 |
| d．Design speed of crossroad |  |  |  | $\square$ | $\square$ | 区 |
| e．Maximum ramp grades： <br> －Exit ramp \％ <br> －Entrance ramp $\qquad$ \％ |  |  |  |  | $\square$ $\square$ | 区 |
| f．Ramp pavement width |  |  |  | $\square$ | $\square$ | 囚 |
| g．Ramp shoulder widths： <br> －Left $\qquad$ feet（meters） <br> －Right $\qquad$ feet（meters） |  |  |  | $\begin{aligned} & \square \\ & \square \end{aligned}$ | $\square$ $\square$ | ® 区 |
| h．Horizontal ramp curvature in conjunction with selected design speeds |  |  |  | $\square$ | $\square$ | 区 |
| i．Superele develop ramps | ation nt on | Supe <br> Trans <br> Distri <br> tange | rate |  | $\square$ $\square$ $\square$ | 区 <br> 区 <br> 囚 |
| j．Vertical curvature compliance with selected design speed on ramp |  |  |  | $\square$ | $\square$ | 区 |
| k．Length of access control at crossroad |  |  |  | $\square$ | $\square$ | 区 |
| I．Type of traffic control at crossroad： <br> －Stop signs <br> －Traffic signals <br> －Free flow |  |  |  | $\square$ $\square$ $\square$ | $\square$ $\square$ $\square$ | $\begin{aligned} & \boxtimes \\ & \boxtimes \\ & \boxtimes \end{aligned}$ |
| m ．Is length of crest vertical curve used on crossroad $\geq$ that required by the selected design speed of crossroad？ |  |  |  | $\square$ | $\square$ | 区 |


| Design Criteria <br> （Provide numerical values，where indicated．） | Does the proposed design meet the criteria？ |  |  |
| :---: | :---: | :---: | :---: |
|  | Yes | No | N／A |
| n．Are crossroad approach grades through ramp／ crossroad intersections $\leq 2 \%$ ？ | $\square$ | $\square$ | 区 |
| o．Are ramp／crossroad intersections located on a tangent section of crossroad alignment？ | $\square$ | $\square$ | 区 |
| p．Is decision sight distance available in advance of exit gore？ | $\square$ | $\square$ | 区 |
| q．Is clear recovery area available beyond gore nose？ | $\square$ | $\square$ | 区 |
| r．Level of service： <br> －Exit terminal <br> －Entrance terminal $\qquad$ <br> －Ramp proper $\qquad$ <br> －Weaving area $\qquad$ <br> －Ramp／crossroad intersection | $\square$ $\square$ $\square$ $\square$ $\square$ |  | $\begin{aligned} & \boxtimes \\ & \boxtimes \\ & \boxtimes \\ & \boxtimes \\ & \boxtimes \end{aligned}$ |
|   Upgrade <br> s．Freeway lane <br> drops Location Inside lane <br> Outside lane <br> At exit <br> terminal <br> Beyond exit <br> terminal <br> Taper length | $\square$ <br> $\square$ <br> $\square$ <br> $\square$ <br> $\square$ <br> $\square$ <br> $\square$ | $\square$ | 区 <br> 区 <br> 区 <br> 区 <br> 区 <br> 区 <br> 区 |
| 7．Roadside Safety（Chapter 38） |  |  |  |
| a．Horizontal clearances： <br> －Clear zones on tangent sections 30 feet <br> －Clear zones on outside of horizontal curves | $\begin{aligned} & \boxtimes \\ & \boxtimes \end{aligned}$ |  | $\begin{aligned} & \square \\ & \square \end{aligned}$ |
| b．Barrier warrants <br> Where clear zone width not met，guardrail was added． | 区 | $\square$ | $\square$ |
| c．Barrier length of need To be designed in Phase II | $\square$ | $\square$ | 区 |


| Design Criteria <br> （Provide numerical values，where indicated．） | Does the proposed design meet the criteria？ |  |  |
| :---: | :---: | :---: | :---: |
|  | Yes | No | N／A |
| d．Deceleration criteria for impact attenuators | $\square$ | $\square$ | 区 |
| 8．Structure Planning／Geometrics（Chapter 39） |  |  |  |
| a．Clear roadway bridge widths feet（meters） | $\square$ | $\square$ | 区 |
| b．Structural capacity of bridges | $\square$ | $\square$ | 区 |
| c．Vertical clearances feet（meters） | $\square$ | $\square$ | 区 |
| 9．Pavement Design（Chapter 54） |  |  |  |
| a．Structural capacity of roadway | $\square$ | $\square$ | ® |

Note：Use multiple forms for each roadway within the project．

Prepared by：


Date：


## Appendix A-2

## Design Exception Forms

| Design Exception Table |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Level One Design Exceptions |  |  |  |  |
| \# | Proposed Design | BDE Standard | Location | Reason for Exception |
| 1 | Lane 1: -2.0\%; <br> Lane 2: +2.0\% | Through Lane Cross <br> Slopes: Lane 1: <br> +2.0\%; Lane 2: <br> +2.0\%; (BDE 34- <br> 2.01(b)); Travel <br> Lane Cross Slope <br> +2.0\% (BDE Figure <br> 46-3E) | IL RTE 47: Sta. 381+00 <br> to $716+00$ | Sloping of inside lanes to the median improves the water quality in an environmentally sensitive area. The median has shoulders and is depressed. Inlets have been placed in the median to minimize the potential of pavement flooding. |
| 2 | $\mathrm{R}=275$-feet | Minimum Radius for Horizontal Curve on Local Street: <br> Rmin=835-feet (50mph Design Speed) (BLRSM Figure 292B) | Pleasant Valley Road: <br> Proposed Curve <br> Pleasant-1; Sta. <br> 2133+27.91 to <br> $2134+49.31$ | Pleasant Valley Road is a local road that will be reconstructed by the local agency at a future date. The proposed 275 -foot curve in necessary in order to avoid the acquisition of a residential property. The proposed curve will be advisory posted for $30-\mathrm{mph}$. Potential mitigation to address the design exception includes advisory posting Pleasant Valley Road for $30-\mathrm{mph}$ in conformance with Rmin=275-feet for $30-\mathrm{mph}$ design speed. |
| 3 | $\mathrm{R}=120$-feet | Minimum Radii for Horizontal Curves on Local Street (BLRSM Figure 292C and Figure 293C): Rmin=125-feet (20-mph Design Speed; emax=4.0\%) | Swanson Road: <br> Proposed Curves: <br> Prswanson-1 Sta. <br> 503+91.66 to <br> $505+27.88$ to <br> 104+00.75; Prswanson- <br> 2 Sta. 505+85.00 to <br> 507+19.86 | Swanson Road is a local street with low traffic volumes. The curves will be advisory posted at $20-\mathrm{mph}$ consistent with the proposed radii of $120-$ feet. Both curves are proximate to a stop condition at its intersection with IL-176. Right-ofway is constrained by a commercial business and by ComEd transmission line towers. Per BRLSM Section 29-4.03(b), e(max) set at 4.0\%. Through lanes are widened to 15 -feet through the curves to accommodate turning vehicles. Potential mitigation to address the design exception includes advisory posting Swanson Road for 20mph consistent with the proposed 120 -foot radii for the curves. |

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| 4 | Distance between PT and PC of Reverse Curves = 57.12-feet | Minimum Tangent Distance Between PT and PC of Reverse Curves on Local Street: for Continuously Rotating Plane = 121.5-feet (BLRSM Equation 29-3.4) | Swanson Road: <br> Proposed Curves: <br> Prswanson-1 Sta. <br> 503+91.66 to <br> $505+27.88$ to <br> 104+00.75; Prswanson- <br> 2 Sta. 505+85.00 to $507+19.86$ | Swanson Road is a local street with low traffic volumes. The curves will be advisory posted at 20-mph consistent with the proposed radii of 120feet. Both curves are proximate to a stop condition at its intersection with IL-176. Right-ofway is constrained by a commercial business and by ComEd transmission line towers. Per BRLSM Section 29-4.03(b), e(max) set at 4.0\%. Through lanes are widened to 15 -feet through the curves to accommodate turning vehicles. Potential mitigation to address the design exception includes advisory posting Swanson Road for 20mph consistent with the proposed 120-foot radii for the curves. |
| :---: | :---: | :---: | :---: | :---: |
| Level 2 Design Exceptions |  |  |  |  |
| \# | Proposed Design | BDE Standard | Location | Reason for Exception |
| 5 | $\mathrm{K}=206$ | Vertical Curve Kvalues > 167 on Curbed Roadways: Maximum K-value for Drainage on Curbed Roadways is 167 ; BDE 33-4.01(d); BDE 33-4.02(e) | IL RTE 47: VPI Sta. $732+85.00$ | Longitudinal profile grades of at least $0.3 \%$ are provided with $2.0 \%$ pavement cross slopes. The potential for travel lane ponding is nonexistent due to the presence of shoulders between the outside travel lanes and the proposed mountable curb and gutters. |
| 6 | $\mathrm{K}=205$ | Vertical Curve Kvalues > 167 on Curbed Roadways: Maximum K-value for Drainage on Curbed Roadways is 167; BDE 33-4.01(d); BDE 33-4.02(e) | IL RTE 47: VPI Sta. 738+00.00 | Longitudinal profile grades of at least $0.3 \%$ are provided with $2.0 \%$ pavement cross slopes. The potential for travel lane ponding is nonexistent due to the presence of shoulders between the outside travel lanes and the proposed mountable curb and gutters. |
| 7 | $\mathrm{K}=168$ | Vertical Curve Kvalues > 167 on Curbed Roadways: Maximum K-value for Drainage on Curbed Roadways is 167; BDE 33-4.01(d); BDE 33-4.02(e) | IL RTE 47: VPI Sta. $742+00.00$ | Longitudinal profile grades of at least $0.3 \%$ are provided with $2.0 \%$ pavement cross slopes. The potential for travel lane ponding is nonexistent due to the presence of shoulders between the outside travel lanes and the proposed mountable curb and gutters. |

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$\left.\begin{array}{|l|l|l|l|l|}\hline 8 & \text { K=188 } & \begin{array}{l}\text { Vertical Curve K- } \\ \text { values > 167 on } \\ \text { Curbed Roadways: } \\ \text { Maximum K-value for } \\ \text { Drainage on Curbed } \\ \text { Roadways is 167; } \\ \text { BDE 33-4.01(d); BDE } \\ 33-4.02(e)\end{array} & \begin{array}{l}\text { IL RTE 47: VPI Sta. } \\ 744+00.00\end{array} & \begin{array}{l}\text { Longitudinal profile grades of at least 0.3\% are } \\ \text { provided with 2.0\% pavement cross slopes. The } \\ \text { potential for travel lane ponding is nonexistent } \\ \text { due to the presence of shoulders between the } \\ \text { outside travel lanes and the proposed mountable } \\ \text { curb and gutters. }\end{array} \\ \hline 9 & \text { 30-feet } & \begin{array}{l}\text { Depressed Rural } \\ \text { Median Width: } \\ \text { Greater Than or } \\ \text { Equal to 40-feet; } \\ \text { (BDE Figure 34-3.A) }\end{array} & \begin{array}{l}\text { IL RTE 47: proposed } \\ \text { rural typical section of } \\ \text { mainline roadway: } \\ \text { Sta. 381+00 to } \\ 617+89 ; \text { Sta. 640+88 } \\ \text { to }\end{array} & \begin{array}{l}\text { 718+00 }\end{array} \\ \hline \text { Policy value impacts other than cost include } \\ \text { additional ROW acquisition that would result in } \\ \text { greater socio-economic and substantial wetland } \\ \text { impacts. Applying the design exception saves } \\ \text { approximately 8-acres of wetland impacts. The } \\ \text { design exception minimizes the levels of } \\ \text { environmental impacts. The proposed 30-foot } \\ \text { rural median matches the existing/proposed 18- } \\ \text { foot urban median and the project termini when } \\ \text { the two 6-foot shoulders are tapered to zero. The } \\ \text { clear width between the innermost northbound } \\ \text { and southbound travel lanes is 30-feet e-e. } \\ \text { Potential mitigation to address the design } \\ \text { exception includes the use of cable barrier } \\ \text { protection. }\end{array}\right\}$

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| 11 | 14 foot V- <br> shaped drainage swales at 6:1 (16.7\%) slopes between back of curb and proposed multi-use path or 5-foot sidewalk | Outside Roadway Ditch: 3 to 10-foot shelves at 5\% cross slopes behind back of curb before sidewalk or drainage swale at variable depths; (BDE Figure 34-4.B) | IL-47: Sta. 718+00 to 745+92.31; and Sta. 752.84 .98 to $773+19.98$ | Policy value impacts other than cost include additional ROW acquisition that would result in greater socio-economic and substantial wetland impacts. The 14 foot V-shaped drainage swales at 6:1 (16.7\%) slopes between back of curb and proposed multi-use path or sidewalk are required to pick up localized drainage. |
| :---: | :---: | :---: | :---: | :---: |
| 12 | 4:1 median slopes | Rural Median Ditch Slopes: 5:1 median slopes; (BDE 343.04(c)) | IL RTE 47: Regions of the mainline roadway where left-turn channelization is present: <br> Sta. $374+68$ to $380+33$ <br> Sta. $393+70$ to $406+40$ <br> Sta. $416+46$ to $428+83$ <br> Sta. $447+35$ to $460+85$ <br> Sta. $473+15$ to $485+70$ <br> Sta. $511+99$ to $524+79$ <br> Sta. $551+74$ to $564+72$ <br> Sta. 569+49 to 581+99 <br> Sta. 589+72 to 606+25 <br> Sta. $627+80$ to $633+40$ <br> Sta. $652+34$ to $665+14$ <br> Sta. $685+10$ to $697+76$ <br> Sta. $720+58$ to $729+35$ <br> Sta. $734+23$ to $742+81$ <br> Sta. $754+90$ to $758+73$ <br> Sta. 763+02 to 772+77 | Policy value impacts other than cost include additional ROW acquisition that would result in greater socio-economic and substantial wetland impacts. Approximately 8-acres of wetland impacts are eliminated. The design exception values minimize the levels of environmental impacts. |
| 13 | Intersection <br> Sight Distance: <br> 845 ft (BDE <br> Figure 36-6.E) | 530 ft | Rainsford Dr. right-turn onto northbound IL Route 47 | Due to the horizontal curve south of this intersection, vehicles turning right from Rainsford Dr. onto northbound IL 47 may not be able to see the approaching vehicles for the entire length of the required ISD. Policy value impacts other than cost include additional ROW acquisition that would result in greater socio-economic impacts. Adjusting the alignment of IL 47 would require additional ROW from commercial properties adjacent to IL 47 south of Rainsford Dr as well as affecting the signalized intersection of IL 47 and Reed Rd to the south. |

## IL Route 47 from Reed Road to US 14 <br> McHenry County <br> Job No. P-91-101-07

| 14 | Intersection <br> Sight Distance: <br> 1350 ft (BDE <br> Figure 36-6.E) | 852 ft | Pleasant Valley Road left-turn onto northbound IL 47 . | Due to the horizontal curve south of this intersection, vehicles turning right from Pleasant Valley Rd. onto northbound IL 47 may not be able to the approaching vehicles for the entire length of the required ISD. Policy value impacts other than cost include additional ROW acquisition that would result in greater socioeconomic and substantial wetland impacts in order to either re-align Pleasant Valley Rd further to the north or to adjust the IL 47 horizontal curve at the intersection of IL 176. |
| :---: | :---: | :---: | :---: | :---: |
| 15 | $\mathrm{K}=180$ | Maximum K - value for Drainage on Curbed Roadways is 167; BDE 33-4.A | IL RTE 47: VPI Sta. $581+60.00$ | The minimum vertical curve length is proposed at this location, which results in a K-value greater than the maximum. The proposed profile is constrained by ROW limitations, existing development and wetlands. The need to have a minimum grade-line of $0.3 \%$ grade for drainage purposes is met within this section. |
| 16 | $\mathrm{K}=190$ | Maximum K - value for Drainage on Curbed Roadways is 167; BDE 33-4.A | IL RTE 47: VPI Sta. $630+00.00$ | The minimum vertical curve length is proposed at this location, which results in a K-value greater than the maximum. The proposed profile is constrained by ROW limitations, existing development and wetlands. The need to have a minimum grade-line of $0.3 \%$ grade for drainage purposes is met within this section. |
| 17 | $\mathrm{K}=64$ | Minimum K - value is 151 for 60 mph design speed; BDE 33-4.B | IL RTE 176 (East <br> Leg): Sta. 302+10.00 | The design of the vertical curve results in a K value less than the minimum. The proposed profile is constrained by ROW limitations, existing development and wetlands. |
| 18 | SSD $=375$ | Minimum sight distance is 570 for 60 mph and level grade; BDE Figure 33-4.B | IL RTE 176 (East Leg): Sta. 302+10.00 | The design of the vertical curve results in a stopping sight distance that is less than the minimum. The proposed profile is constrained by ROW limitations, existing development and wetlands. |
| 19 | $\mathrm{K}=187$ | Maximum K - value for Drainage on Curbed Roadways is 167; BDE 33-4.A | IL RTE 176 (West Leg): Sta. 416+80.00 | The design of the vertical curve results in a K value less than the minimum. The proposed profile is constrained by ROW limitations, existing development and wetlands. |

## IL Route 47 from Reed Road to US 14 <br> McHenry County <br> Job No. P-91-101-07

| 20 | K=69 | Minimum K - value is <br> 151 for 60 mph <br> design speed; BDE <br> $33-4 . \mathrm{B}$ | IL RTE 176 (West <br> Leg): Sta. 426+75.00 | The design of the vertical curve results in a <br> stopping sight distance that is less than the <br> minimum. The proposed profile is constrained by <br> ROW limitations, existing development and <br> wetlands. |
| :--- | :--- | :--- | :--- | :--- |
| 21 | 2.7 feet | 3.0 feet | Sta. $398+00$ |  <br> wildlife impacts. Increasing the proposed <br> structure size will not increase freeboard value <br> due to natural high-water elevation. There are no <br> reports of flooding at this location. 100-year edge <br> of pavement protection will be provided. Sloping <br> of inside lanes to the median improves the water <br> quality in an environmentally sensitive area. The <br> median has shoulders and is depressed. Inlets <br> have been placed in the median to minimize the <br> potential of pavement flooding. |


| Route | Street | Marked | Contract \# | State Job \# |
| :---: | :---: | :---: | :---: | :---: |
| FAP 0326 |  | Illinois Route 47 |  | P-91-101-07 |
| Section | County |  | Municipality |  |
|  | McHenry |  | **Grafton Township and Dorr Township |  |
| Local Agency | LRS Section \# | Permit Applicant |  | ermit \# |
| N/A | N/A | N/A |  | /A |


| Reed Road to US-14 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Project Length | Current Posted Speed |  |  | FHWA Oversight? |
| 7.6-miles (40,425-feet) |  |  | 40-55-mph | $\square$ Yes $\quad$ No |
| Estimate of Cost Functional Classification | Design Yr Design Traffic ADT Design Traffic DHV |  |  |  |
| 102,235,894 Other Princ. Arterial (SRA) | 2040 | 28,000 | AM 2,105 | PM 2,250 |
| On the NHS System? Structure Numbers | Type of Project (Construction, Reconstruction, 3R, HES, etc.) |  |  |  |
| $\square$ Yes X No 056-0025 | Reconstruction |  |  |  |

## Brief Project Description

Reconstruction of IL-47 from 2-lane undivided to a 4-lane divided highway.

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non-Interstate
Design Element for Which an Exception is Requested
Through Lane Cross Slopes
Design Element Policy Value
Lane 1: +2.0\%; Lane 2: +2.0\%; Lane 3+: +2.5\% - BDE 34-2.01(b); 2\% for Two Lanes Adjacent to Median - BDE Figure 46-3E

## Proposed Design Element Value

Lane 1: -2.0\%; Lane 2: +2.0\%
Location(s) of Exception
Sta. 381+00 to 716+00
Crash History and Potential of Exception Location(s)
368 total crashes between 2008 and 2012. None were related to this Design Exception. It was not a 5\% segment for 2008 to 2012; however, the split intersection of IL176 was a $5 \%$ location in 2010. Proposed scope of work will improve the safety and operations related to this exception request.
Cost of Using Policy Value

## Cost of Using Proposed Exception Value

$\$ 0.00$

## Impacts Other Than Cost of Using Policy Value

Sloping of inside lanes to the median improves the water quality in an environmentally sensitive area.

## Proposed Mitigation to Address Exception

Use of a skid-resistant pavement surfaces in areas of cross slope design variances; Inlet spacing designed to ensure no standing water encroachments in areas of cross slope variances.
Geometric Compatibility with Adjacent Sections
Proposed design elements transition and match the proposed typical sections of the IL-47 project.

## Potential Effects on Other Design Elements

None


| Route | Street | Marked | Contract \# | State Job \# |
| :---: | :---: | :---: | :---: | :---: |
| FAP 0326 |  | Illinois Route 47 |  | P-91-101-07 |
| Section | County |  | Municipality |  |
|  | McHenry |  | **Grafton Township and Dorr Township |  |
| Local Agency | LRS Section \# | Permit Applicant |  | ermit \# |
| N/A | N/A | N/A |  | /A |


| Reed Road to US-14 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Project Length |  |  | Current Posted Speed | FHWA Oversight? |  |
| 7.6-miles (40,425-feet) |  |  | 40-55-mph | $\square$ Yes | \ No |
| Estimate of Cost Functional Classification | Design Yr Design Traffic ADT |  | Design Traffic DHV |  |  |
| 102,235,894 Other Princ. Arterial (SRA) | 2040 | 5,000 | AM 380 | PM 345 |  |
| On the NHS System? Structure Numbers | Type of Project (Construction, Reconstruction, 3R, HES, etc.) |  |  |  |  |
| $\square$ Yes $\boxtimes$ No 056-0025 | Reconstruction |  |  |  |  |

## Brief Project Description

Reconstruction of IL-47 from 2-lane undivided to a 4-lane divided highway.

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non-Interstate
Design Element for Which an Exception is Requested
Minimum Radius for Horizontal Curve on Local Street
Design Element Policy Value
Rmin=835-feet for 50-mph Design Speed and e(max)=6.0\%; BLRSM Figure 29-2B

## Proposed Design Element Value

## R=275-feet

## Location(s) of Exception

Proposed Curve Pleasant-1; Sta. 2133+27.91 to 2134+49.31
Crash History and Potential of Exception Location(s)
368 total crashes between 2008 and 2012. None were related to this Design Exception. It was not a $5 \%$ location for 2008 to 2012. Proposed scope of work will improve the safety and operations related to this exception request.

## Cost of Using Policy Value

## Cost of Using Proposed Exception Value

Impacts Other Than Cost of Using Policy Value
None

## Proposed Mitigation to Address Exception

Curve will be advisory posted for 30-mph in conformance with Rmin=275-feet for 30-mph design speed.

## Geometric Compatibility with Adjacent Sections

Proposed curve is geometrically compatible with the existing horizontal alignment.

## Potential Effects on Other Design Elements

None


| Route | Street | Marked | Contract \# | State Job \# |
| :---: | :---: | :---: | :---: | :---: |
| FAP 0326 |  | Illinois Route 47 |  | P-91-101-07 |
| Section | County |  | Municipality |  |
|  | McHenry |  | **Grafton Township and Dorr Township |  |
| Local Agency | LRS Section \# | Permit Applicant |  | ermit \# |
| N/A | N/A | N/A |  | /A |

## Project Limits

| Reed Road to US-14 |  |
| :--- | :--- |
| Project Length |  |
| 7.6-miles (40,425-feet) |  |
| Estimate of Cost | Functional Classification |
| $102,235,894$ | Other Princ. Arterial (SRA) |


| On the NHS System? | Structure Numbers |
| :--- | :--- |
| $\square$ Yes $\boxtimes$ No | $056-0025$ |


|  |  | Current Posted Speed |  | FHWA Oversight? |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 40-55 |  | $\square$ Yes | 区 No |
| Design Yr | Design Traffic ADT |  |  | affic DHV |  |
| 2040 |  |  | AM 40 | PM 20 |  |

## Brief Project Description

Reconstruction of IL-47 from 2-lane undivided to a 4-lane divided highway.

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non-Interstate
Design Element for Which an Exception is Requested
Minimum Radii for Horizontal Curves on Local Street
Design Element Policy Value
Low Speed Urban (BLRSM 29-4): Rmin=125-feet for 20-mph Design Speed and e(max)=4.0\% - BLRSM Figure 29-3C
Proposed Design Element Value
$R=120$-feet for 20-mph Design Speed and e(max)=4.0\%

## Location(s) of Exception

Proposed Curves: Prswanson-1 Sta. 503+91.66 to 505+27.88; Prswanson-2 Sta. 505+85.00 to 507+19.86

## Crash History and Potential of Exception Location(s)

368 total crashes between 2008 and 2012. None were related to this Design Exception. It was not a 5\% location for 2008 to 2012. Proposed scope of work will improve the safety and operations related to this exception request.

Cost of Using Policy Value

## Cost of Using Proposed Exception Value

\$2,000,000.00

Type of Project (Construction, Reconstruction, 3R, HES, etc.)
Reconstruction

Impacts Other Than Cost of Using Policy Value
None

## Proposed Mitigation to Address Exception

Curve will be advisory posted at 20-mph consistent with the proposed 120-foot radii for the curves.
Geometric Compatibility with Adjacent Sections
Proposed curve is compatible with adjacent sections.

## Potential Effects on Other Design Elements

None
Potential Impacts on Mobility or Traffic Operations
None

## Summary of Justification for Exception

Right-of-way is constrained by a commercial business and by ComEd power line towers.

| Coordination Meeting Date |
| :--- |


| Route | Street | Marked | Contract \# | State Job \# |
| :---: | :---: | :---: | :---: | :---: |
| FAP 0326 |  | Illinois Route 47 |  | P-91-101-07 |
| Section | County |  | Municipality |  |
|  | McHenry |  | **Grafton Township and Dorr Township |  |
| Local Agency | LRS Section \# | Permit Applicant |  | ermit \# |
| N/A | N/A | N/A |  | /A |



## Brief Project Description

Reconstruction of IL-47 from 2-lane undivided to a 4-lane divided highway.

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non-Interstate
Design Element for Which an Exception is Requested
Minimum Radii for Horizontal Curves on Local Street
Design Element Policy Value
Low Speed Urban (BLRSM 29-4): Minimum Tangent Distance Between PT and PC of Reverse Curves for Continuously Rotating Plane of 121.5-feet - BLRSM Equation 29-3.4

## Proposed Design Element Value

Distance between PT and PC of Reverse Curves=57.12-feet

## Location(s) of Exception

Proposed Curves: Prswanson-1 Sta. 503+91.66 to 505+27.88; Prswanson-2 Sta. 505+85.00 to 507+19.86

## Crash History and Potential of Exception Location(s)

368 total crashes between 2008 and 2012. None were related to this Design Exception. It was not a 5\% location for 2008 to 2012. Proposed scope of work will improve the safety and operations related to this exception request.

Cost of Using Policy Value
\$2,000,000.00

## Cost of Using Proposed Exception Value

 $\$ 0.00$Impacts Other Than Cost of Using Policy Value
None

## Proposed Mitigation to Address Exception

Curve will be advisory posted at 20-mph consistent with the proposed 120-foot radii for the curves.
Geometric Compatibility with Adjacent Sections
Proposed curve is compatible with adjacent sections.

## Potential Effects on Other Design Elements

None

Potential Impacts on Mobility or Traffic Operations

## None

Summary of Justification for Exception
Right-of-way is constrained by a commercial business and by ComEd power line towers.


Location(s) of Exception

|  |  |
| :--- | :--- |
| Cost of Using Policy Value | Cost of Using Proposed Element Value |
|  | $\square$ |
| Summary of Justification |  |
|  | Date |
| Prepared By | APPROVAL/DISAPPROVAL |
| BDE Approval Date | BDE Disapproval Date |
|  |  |
| BDE Comments on Disapproval |  |
|  |  |
| FHWA Approval Date (Interstate Only) | FHWA Disapproval Date (Interstate Only) |
|  |  |



| Reed Road to US－14 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Project Length |  |  | Current Posted Speed | FHWA Oversight？ |  |
| 7．6－miles（40，425－feet） |  |  | 40－55－mph | $\square$ Yes | \ No |
| Estimate of Cost Functional Classification | Design Yr Design Traffic ADT |  | Design Traffic DHV |  |  |
| 102，235，894 Other Princ．Arterial（SRA） | 2040 | 26，000 | AM 1，615 | PM 1，905 |  |
| On the NHS System？Structure Numbers | Type of Project（Construction，Reconstruction，3R，HES，etc．） |  |  |  |  |
| $\square$ Yes $\boxtimes$ No 056－0025 | Reconstruction |  |  |  |  |

## Brief Project Description

Reconstruction of IL－47 from 2－lane undivided to a 4－lane divided highway．

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non－Interstate
Design Element for Which an Exception is Requested
Vertical Curve K－values＞ 167 on Curbed Roadways

## Design Element Policy Value

Maximum K－value for Drainage on Curbed Roadways is 167；BDE 33－4．01（d）；BDE 33－4．02（e）

## Proposed Design Element Value

K＝206 L＝165－feet

## Location（s）of Exception

VPI Sta．732＋85．00
Crash History and Potential of Exception Location（s）
368 total crashes between 2008 and 2012．None were related to this Design Exception．It was not a $5 \%$ location for 2008 to 2012．Proposed scope of work will improve the safety and operations related to this exception request．
Cost of Using Policy Value

## Cost of Using Proposed Exception Value

```
\(\$ 0.00\)
```

Impacts Other Than Cost of Using Policy Value
None

## Proposed Mitigation to Address Exception

Shoulders adjacent to outside travel lanes and mountable curb and gutter．

## Geometric Compatibility with Adjacent Sections

Proposed design elements transition and match the proposed typical sections of the IL－47 project．

## Potential Effects on Other Design Elements

None



| Reed Road to US－14 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Project Length |  |  | Current Posted Speed | FHWA Oversight？ |  |
| 7．6－miles（40，425－feet） |  |  | 40－55－mph | $\square$ Yes | \ No |
| Estimate of Cost Functional Classification | Design Yr Design Traffic ADT |  | Design Traffic DHV |  |  |
| 102，235，894 Other Princ．Arterial（SRA） | 2040 | 26，000 | AM 1，615 | PM 1，905 |  |
| On the NHS System？Structure Numbers | Type of Project（Construction，Reconstruction，3R，HES，etc．） |  |  |  |  |
| $\square$ Yes $\boxtimes$ No 056－0025 | Reconstruction |  |  |  |  |

## Brief Project Description

Reconstruction of IL－47 from 2－lane undivided to a 4－lane divided highway．

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non－Interstate
Design Element for Which an Exception is Requested
Vertical Curve K－values＞ 167 on Curbed Roadways

## Design Element Policy Value

Maximum K－value for Drainage on Curbed Roadways is 167；BDE 33－4．01（d）；BDE 33－4．02（e）

## Proposed Design Element Value

K＝205 L＝165－feet

## Location（s）of Exception

VPI Sta．738＋00．00
Crash History and Potential of Exception Location（s）
368 total crashes between 2008 and 2012．None were related to this Design Exception．It was not a $5 \%$ location for 2008 to 2012．Proposed scope of work will improve the safety and operations related to this exception request．
Cost of Using Policy Value

## Cost of Using Proposed Exception Value

Impacts Other Than Cost of Using Policy Value
None

## Proposed Mitigation to Address Exception

Shoulders adjacent to outside travel lanes and mountable curb and gutter．

## Geometric Compatibility with Adjacent Sections

Proposed design elements transition and match the proposed typical sections of the IL－47 project．

## Potential Effects on Other Design Elements

None




## Brief Project Description

Reconstruction of IL－47 from 2－lane undivided to a 4－lane divided highway．

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non－Interstate
Design Element for Which an Exception is Requested
Vertical Curve K－values＞ 167 on Curbed Roadways

## Design Element Policy Value

Maximum K－value for Drainage on Curbed Roadways is 167；BDE 33－4．01（d）；BDE 33－4．02（e）

## Proposed Design Element Value

## K＝168 L＝135－feet

## Location（s）of Exception

VPI Sta．742＋00．00
Crash History and Potential of Exception Location（s）
368 total crashes between 2008 and 2012．None were related to this Design Exception．It was not a $5 \%$ location for 2008 to 2012．Proposed scope of work will improve the safety and operations related to this exception request．
Cost of Using Policy Value

## Cost of Using Proposed Exception Value

\＄200，000．00

```
\(\$ 0.00\)
```

Impacts Other Than Cost of Using Policy Value
None

## Proposed Mitigation to Address Exception

Shoulders adjacent to outside travel lanes and mountable curb and gutter．

## Geometric Compatibility with Adjacent Sections

Proposed design elements transition and match the proposed typical sections of the IL－47 project．

## Potential Effects on Other Design Elements

None




## Brief Project Description

Reconstruction of IL－47 from 2－lane undivided to a 4－lane divided highway．

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non－Interstate
Design Element for Which an Exception is Requested
Vertical Curve K－values＞ 167 on Curbed Roadways
Design Element Policy Value
Maximum K－value for Drainage on Curbed Roadways is 167；BDE 33－4．01（d）；BDE 33－4．02（e）
Proposed Design Element Value
K＝188 L＝150－feet
Location（s）of Exception
VPI Sta．744＋00．00
Crash History and Potential of Exception Location（s）
368 total crashes between 2008 and 2012．None were related to this Design Exception．It was not a 5\％location for 2008 to 2012．Proposed scope of work will improve the safety and operations related to this exception request．
Cost of Using Policy Value

## Cost of Using Proposed Exception Value

```
\(\$ 0.00\)
```

Impacts Other Than Cost of Using Policy Value
None

## Proposed Mitigation to Address Exception

Shoulders adjacent to outside travel lanes and mountable curb and gutter．

## Geometric Compatibility with Adjacent Sections

Proposed design elements transition and match the proposed typical sections of the IL－47 project．

## Potential Effects on Other Design Elements

None


| Route | Street | Marked | Contract \# | State Job \# |
| :---: | :---: | :---: | :---: | :---: |
| FAP 0326 |  | Illinois Route 47 |  | P-91-101-07 |
| Section | County |  | Municipality |  |
|  | McHenry |  | **Grafton Township and Dorr Township |  |
| Local Agency | LRS Section \# | Permit Applicant |  | ermit \# |
| N/A | N/A | N/A |  | /A |


| Reed Road to US-14 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Project Length | Current Posted Speed |  |  | FHWA Oversight? |
| 7.6-miles (40,425-feet) |  |  | 40-55-mph | $\square$ Yes $\quad$ No |
| Estimate of Cost Functional Classification | Design Yr Design Traffic ADT Design Traffic DHV |  |  |  |
| 102,235,894 Other Princ. Arterial (SRA) | 2040 | 28,000 | AM 2,105 | PM 2,250 |
| On the NHS System? Structure Numbers | Type of Project (Construction, Reconstruction, 3R, HES, etc.) |  |  |  |
| $\square$ Yes X No 056-0025 | Reconstruction |  |  |  |

## Brief Project Description

Reconstruction of IL-47 from 2-lane undivided to a 4-lane divided highway.

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non-Interstate
Design Element for Which an Exception is Requested
Depressed Rural Median Width
Design Element Policy Value
Greater Than or Equal to 40-feet; BDE Figure 34-3.A

## Proposed Design Element Value

30-feet

## Location(s) of Exception

Proposed rural typical section of mainline roadway; Sta. 381+00 to 718+00.

## Crash History and Potential of Exception Location(s)

368 total crashes between 2008 and 2012. None were related to this Design Exception. It was not a 5\% segment for 2008 to 2012; however, the split intersection of IL176 was a $5 \%$ location in 2010. Proposed scope of work will improve the safety and operations related to this exception request.

## Cost of Using Policy Value

## Cost of Using Proposed Exception Value

\$5,000,000.00 $\$ 0.00$

## Impacts Other Than Cost of Using Policy Value

Policy value impacts other than cost for full width medians include additional ROW acquisition that would result in greater socio-economic and wetland impacts. The design exception values minimize the levels of environmental impacts.

## Proposed Mitigation to Address Exception

## Use of Cable Barrier

Geometric Compatibility with Adjacent Sections
Matches existing and proposed 18-foot urban median at project termini when the shoulders are tapered to zero feet.

## Potential Effects on Other Design Elements

[^0]
## None

Summary of Justification for Exception
Policy value impacts other than cost include additional ROW acquisition that would result in greater socio-economic and substantial wetland impacts. Applying the design exception saves approximately 8-acres of wetland impacts. The design exception minimizes the levels of environmental impacts.


| Location(s) of Exception |  |
| :--- | :--- |
|  |  |
| Cost of Using Policy Value | Cost of Using Proposed Element Value |
|  |  |
| Summary of Justification |  |
|  |  |
| Prepared By | Date |
|  |  |
|  | BDE Disapproval Date |
|  |  |
| BDE Approval Date |  |
|  |  |
| FHWA Approval Date (Interstate Only) | FHWA Disapproval Date (Interstate Only) |
|  |  |




## Brief Project Description

Reconstruction of IL-47 from 2-lane undivided to a 4-lane divided highway.

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non-Interstate
Design Element for Which an Exception is Requested
Left Turn Lane Storage Lengths
Design Element Policy Value
185-foot minimum storage length for 45-mph design speed; BDE Figure 36-3.I

## Proposed Design Element Value

Back-to-back left turn storage bays of 137.4-feet separated by 200' taper.

## Location(s) of Exception

IL-47 - between US-14 and Willow Brooke Drive; Sta. 768+01 to 772+77
Crash History and Potential of Exception Location(s)
368 total crashes between 2008 and 2012. Two were related to this Design Exception. It was not a 5\% location for 2008 to 2012. Proposed scope of work will improve the safety and operations related to this exception request.

## Cost of Using Policy Value

## Cost of Using Proposed Exception Value

\$500,000.00

```
\(\$ 0.00\)
```

Impacts Other Than Cost of Using Policy Value
Eliminating the southbound to eastbound left turn lane to Willow Brooke Drive, in favor of applying the full design requirements to the northbound to westbound left turn at US-14 would slightly modify traffic patterns in the area, and create a public inconvenience for those users accustomed to full access at Willow Brooke Drive.

## Proposed Mitigation to Address Exception

Effective storage of the left turn bays are 204-feet which includes one-third of the 200-foot taper length (66.7-feet). The opening to the left turn storage bays are 8 -feet in width at a distance of 66.7 -feet from the end of the storage bays. The effective storage of 204 -feet meets the minimum 185-foot requirement for the stop condition.

## Geometric Compatibility with Adjacent Sections

Proposed design elements transition and match the proposed typical sections of the IL-47 project.

## Potential Effects on Other Design Elements

None

## None

Summary of Justification for Exception
It is physically impossible to maintain access to existing roadways and maintain minimum distance between the storage bays due to the location of the existing roadways. Eliminating the southbound to eastbound left turn lane to Willow Brooke Drive, in favor of applying the full design requirements to the northbound to westbound left turn at US-14 would modify traffic patterns in the area, and create a public inconvenience for those users accustomed to full access at Willow Brooke Drive.

| Coordination Meeting Date | Prepared By | Date |
| :--- | :--- | :--- |
|  | Kirsten Mawhinney, P.E., AECOM | 12/24/2015 |
|  |  |  |

## PAVEMENT/RESURFACING EXCEPTIONS

| $\square$ Pavement Widening $\square$ Resurfacing |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Design Period/ Expected Service Life | Design Year | Structural Design Traffic | \%PV | \%SU | \%MU |
|  |  |  |  |  |  |
| Design Element Policy Value | Proposed Design Element Value |  |  |  |  |
| Location(s) of Exception |  |  |  |  |  |
| Cost of Using Policy Value | Cost of Using Proposed Element Value |  |  |  |  |
| Summary of Justification |  |  |  |  |  |
| Prepared By | Date |  |  |  |  |
| APPROVALIDISAPPROVAL |  |  |  |  |  |
| BDE Approval Date | BDE Disapproval Date |  |  |  |  |
| BDE Comments on Disapproval |  |  |  |  |  |
| FHWA Approval Date (Interstate Only) | FHWA Disapproval Date (Interstate Only) |  |  |  |  |


| Route | Street |  | Marked | Contract \# | State Job \# |
| :---: | :---: | :---: | :---: | :---: | :---: |
| FAP 0326 |  |  | Illinois Route 47 |  | P-91-101-07 |
| Section County |  |  |  | Municipality |  |
| McHenry |  |  |  | **Grafton Township and Dorr Township |  |
| Local Agency LRS Section \# |  |  | Permit Applicant |  | Permit \# |
| N/A |  | N/A | N/A |  | N/A |
| Project Limits |  |  |  |  |  |
| Reed Road to US-14 |  |  |  |  |  |
| Project Length |  |  | Current Posted Speed |  | FHWA Oversight? |
| 7.6-miles (40,425-feet) |  |  | 40-55-mph |  | $\square$ Yes $\quad$ No |
| Estimate of Cost | Functional Classification | Design Yr Design Traffic ADT |  | Design Traffic DHV |  |
| 102,235,894 | Other Princ. Arterial (SRA) | 2040 | 27,000 | AM 1,595 | PM 2,025 |
| On the NHS System?$\square$$\square$ Yes $\triangle$ No |  | Type of Project (Construction, Reconstruction, 3R, HES, etc.) |  |  |  |
|  |  | Reconstruction |  |  |  |

## Brief Project Description

Reconstruction of IL-47 from 2-lane undivided to a 4-lane divided highway.

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non-Interstate
Design Element for Which an Exception is Requested
Outside Roadway Fill Section Shelf and Drainage Swale for Curbed Facilities
Design Element Policy Value
3 to 10 -foot shelves at $5 \%$ cross slopes behind back of curb before sidewalk or drainage swale at variable depths; BDE Figure 34-4.B

## Proposed Design Element Value

14 foot V-shaped drainage swales at 6:1 (16.7\%) slopes between back of curb and proposed multi-use path or 5-foot sidewalk Location(s) of Exception
IL-47: Sta. 718+00 to 745+92.31; and Sta. 752.84.98 to 773+19.98
Crash History and Potential of Exception Location(s)
368 total crashes between 2008 and 2012. None were related to this Design Exception. It was not a 5\% location for 2008 to 2012. Proposed scope of work will improve the safety and operations related to this exception request.

Cost of Using Policy Value
\$500,000.00

## Cost of Using Proposed Exception Value

$\$ 0.00$

## Impacts Other Than Cost of Using Policy Value

Policy value impacts other than cost include additional ROW acquisition that would result in greater socio-economic and substantial wetland impacts. The design exception values minimize the levels of environmental impacts.
Proposed Mitigation to Address Exception

## None

Geometric Compatibility with Adjacent Sections
Proposed design elements transition and match the proposed typical sections of the IL-47 project.

## Potential Effects on Other Design Elements

## None

## None

Summary of Justification for Exception
Policy value impacts other than cost include additional ROW acquisition that would result in greater socio-economic and substantial wetland impacts. The 14 foot $V$-shaped drainage swales at 6:1 (16.7\%) slopes between back of curb and proposed multi-use path or sidewalk are required to pick up localized drainage.



## Brief Project Description

Reconstruction of IL－47 from 2－lane undivided to a 4－lane divided highway．

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non－Interstate
Design Element for Which an Exception is Requested
Rural Median Ditch Bottom Widths and Median Slopes．

## Design Element Policy Value

2－foot ditch bottom widths，5：1 median slopes，variable depths；BDE 34－3．04（C）

## Proposed Design Element Value

2－foot ditch bottom widths，and 4：1 and steeper／variable slopes，variable depths

## Location（s）of Exception

Sta．374＋68 to 380＋33
Sta． $393+70$ to $406+40$
Sta． $416+46$ to $428+83$
Sta． $447+35$ to $460+85$
Sta． $473+15$ to $485+70$
Sta．511＋99 to 524＋79
Sta． $551+74$ to $564+72$
Sta．569＋49 to 581＋99
Sta．589＋72 to 606＋25
Sta． $627+80$ to $633+40$
Sta．652＋34 to 665＋14
Sta．685＋10 to 697＋76
Sta． $720+58$ to $729+35$
Sta．734＋23 to 742＋81
Sta． $754+90$ to $758+73$
Sta．763＋02 to 772＋77

## Crash History and Potential of Exception Location（s）

368 total crashes between 2008 and 2012．None were related to this Design Exception．It was not a 5\％location for 2008 to 2012．Proposed scope of work will improve the safety and operations related to this exception request．

## Cost of Using Policy Value

\＄1，000，000．00

| Cost of Using Proposed Exception Value |
| :--- | :--- |
| $\$ 0.00$ |

Impacts Other Than Cost of Using Policy Value
Policy value impacts other than cost include additional ROW acquisition that would result in greater socio-economic and substantial wetland impacts. The design exception values minimize the levels of environmental impacts.

## Proposed Mitigation to Address Exception

Rounding of ditch bottoms to facilitate conveyance. Geo treatments and Bio-swaling should reduce velocities and erosion.

## Geometric Compatibility with Adjacent Sections

Transitions to match proposed typical section in areas where channelization is not present.

## Potential Effects on Other Design Elements

## None

Potential Impacts on Mobility or Traffic Operations
None

## Summary of Justification for Exception

Policy value impacts other than cost include additional ROW acquisition that would result in greater socio-economic and substantial wetland impacts. Approximately 8-acres of wetland impacts are eliminated. The design exception values minimize the levels of environmental impacts. Proposed ditch depths are 2-feet with that depth needed for water storage of the bio swales.



| Reed Road to US-14 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Project Length | Current Posted Speed |  |  | FHWA Oversight? |  |
| 7.6-miles (40,425-feet) | 40-55-mph |  |  | $\square$ Yes | Х No |
| Estimate of Cost Functional Classification | Design Yr Design Traffic ADT Design Traffic DHV |  |  |  |  |
| 102,235,894 Other Princ. Arterial (SRA) | 2040 | 2,000 | AM 260 | PM 360 |  |
| On the NHS System? Structure Numbers | Type of Project (Construction, Reconstruction, 3R, HES, etc.) |  |  |  |  |
| $\square$ Yes $\boxtimes$ No 056-0025 | Reconstruction |  |  |  |  |

## Brief Project Description

Reconstruction of IL-47 from 2-lane undivided to a 4-lane divided highway.

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non-Interstate
Design Element for Which an Exception is Requested
Intersection Sight Distance for right-turning vehicle from minor street to major street
Design Element Policy Value
845 feet per BDE Figure 36-6.E

## Proposed Design Element Value

## 530 feet

## Location(s) of Exception

Rainsford Dr. right-turn to northbound IL 47

## Crash History and Potential of Exception Location(s)

368 total crashes between 2008 and 2012. None were related to this Design Exception. It was not a 5\% location for 2008 to 2012. Proposed scope of work will improve the safety and operations related to this exception request.

## Cost of Using Policy Value

## Cost of Using Proposed Exception Value

\$1,000,000.00
$\$ 0.00$

## Impacts Other Than Cost of Using Policy Value

Policy value impacts other than cost include additional ROW acquisition that would result in greater socio-economic impacts.
The design exception values minimize the levels of environmental impacts and avoids changes to the section of IL 47 that has already been widened.

## Proposed Mitigation to Address Exception

## None

Geometric Compatibility with Adjacent Sections
Proposed design elements transition and match the existing reconstruction of IL 47 south of Rainsford Dr.

## Potential Effects on Other Design Elements

None



## Brief Project Description

Reconstruction of IL-47 from 2-lane undivided to a 4-lane divided highway.

## EXCEPTION DOCUMENTATION

Level of Exception $\square$ Interstate $\boxtimes$ Non-Interstate
Design Element for Which an Exception is Requested
Intersection Sight Distance for right-turning vehicle from minor street to major street
Design Element Policy Value
1350 feet per BDE Figure 36-6.E
Proposed Design Element Value
852 feet

## Location(s) of Exception

Pleasant Valley Rd. left-turn to northbound IL 47
Crash History and Potential of Exception Location(s)
368 total crashes between 2008 and 2012. None were related to this Design Exception. It was not a 5\% location for 2008 to 2012. Proposed scope of work will improve the safety and operations related to this exception request.

## Cost of Using Policy Value

## Cost of Using Proposed Exception Value

Impacts Other Than Cost of Using Policy Value
Additional right-of-way would be required and additional impacts to wetlands adjacent to the corridor.

## Proposed Mitigation to Address Exception

Re-alignment of Pleasant Valley Rd as part of proposed improvements increase ISD compared to existing.

## Geometric Compatibility with Adjacent Sections

Proposed design elements transition and match the proposed typical sections of the IL-47 project.

## Potential Effects on Other Design Elements

## None




Potential Impacts on Mobility or Traffic Operations
None
Summary of Justification for Exception
The minimum vertical curve length is proposed at this location, which results in a K-value greater than the maximum. The proposed profile is constrained by ROW limitations, existing development and wetlands. The need to have a minimum gradeline of $0.3 \%$ grade for drainage purposes is met within this section.

| Coordination Meeting Date | Prepared By | Date |
| :--- | :--- | :--- |
|  | Kirsten Mawhinney, P.E., AECOM | 09-12-2016 |

PAVEMENT/RESURFACING EXCEPTIONS

| $\square$ New Pavement $\quad \square$ Pavement Widening $\square$ Resurfacing |
| :--- |
| Design Period/ Expected Service Life |









Potential Impacts on Mobility or Traffic Operations
None
Summary of Justification for Exception
The minimum vertical curve length is proposed at this location, which results in a K-value greater than the maximum. The proposed profile is constrained by ROW limitations, existing development and wetlands. The need to have a minimum gradeline of $0.3 \%$ grade for drainage purposes is met within this section.






FHWA Approval Date (Level One)

## Appendix A-3

 Bicycle and Pedestrian Checklist| Generators | Yes | NA | Generators | Yes | NA |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Residential Areas | \% | $\square$ | Shopping Centers | $\square$ | + |
| Parks | \% | $\square$ | Hospitals | $\square$ | , |
| Recreation Areas | * | $\square$ | Employment Center | $\square$ | * |
| Churches | $\square$ | * | Government Offices | $\square$ | \% |
| Schools | t | $\square$ | Local Businesses | * | $\square$ |
| Libraries | $\square$ | * | Industrial Plants | \% | $\square$ |
| Existing Bicycle Trails | t | $\square$ | Public Transportation Facilities | $\square$ | * |
| Planned Bicycle Trails | 大 | $\square$ | Other ( | $\square$ | * |

CHECKLIST FOR BICYCLE TRAVEL GENERATORS IN PROJECT VICINITY
Figure 17-1.A

| Organization | Yes | NA | Organizations* | Yes | NA |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Metropolitan Planning Organization (if applicable) | $A$ | $\square$ | League of Illinois Bicyclists* | $A$ | $\square$ |
| Local Municipalities | * | $\square$ | Illinois Department of Natural Resources* | $x$ | $\square$ |
| Park or Forest Preserve Districts | * | $\square$ | Trails for Illinois* | $\square$ | $A$ |
| Sub-Regional Planning Council (as appropriate) | R | $\square$ | Active Transportation Alliance (District 1 only) ${ }^{*}$ | $\square$ | tr |
| Local Bicycle Clubs, Advocacy Groups |  | $\square$ |  |  |  |

*Note: Addresses are presented in Section 17-5.
CHECKLIST FOR ORGANIZATIONS AND PUBLIC COORDINATION
Figure 17-1.C

## Appendix A-4 <br> TRAFFIC DIAGRAM






## Appendix A-5

## Intersection Design Studies











## Appendix A-6 <br> Alignment Plan






## Appendix A-7

 Typical Cross Sections

acceleration lane
NORMAL CROWN: STA $617+88.61$ TO STA $627+02.93$ 5.80\% TO NORMAL CROWN: STA $627+02.93$ TO $629+10.93$ FULLY SUPERELEVATED @ ( $5.80 \%$ ): STA $629+10.93$ TO STA $633+04.64$

IL 47 WITH ACCELERATION LANE STA $617+88.61$ TO STA $640+87.96$

IL 47
NORNAL CROWN TO 6.00\%: STA 625+92.43 TO STA 629+12.43 6.00\% TO NORMAL CROWN: STA $638+78.46$ TO $641+98.46$







| FILE NAME =39027_SHT_TYPSEC-07.dgn | USER NNME = horegroofe | DESIONED | ${ }_{\text {AFC }}$ | REVISED | State of illinois department of transportation | TYPICAL CROSS SECTIONS BALLARD ROAD |  |  |  |  |  | [EAAP. | Section |  | counir |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | PLOT SCOLE : 20.000 | ${ }_{\text {ORAWN }}^{\text {CHECKED }}$ | ${ }_{\text {KM }}^{\text {PM }}$ | $\underset{\substack{\text { ReVIISED } \\ \text { REVISED }}}{ }$ |  |  |  |  |  |  |  |  |  | $\begin{array}{\|l\|l\|} \hline \text { MCHENRY } & 336 \\ \hline \text { CONTRACT NO. } \\ \hline \end{array}$ |  |  |  |
|  | Plot date $=4 / 5 / 2 / 2017$ | Date |  | REVISED |  | SCALE: | SHEET No. | OF | SHEETS STA. | то | A. |  |  |  |  |  |  |



## Appendix A-8

 Plan and Profile










































## Appendix A-9

Kishwaukee River Bridge Replacement



Appendix A-10

## Traffic Management Plan

## TRAFFIC MANGAGEMENT PLAN

## IL-47 Corridor Improvement Project

 Reed Road to US-14McHenry County, State of Illinois

Section No.: TBD


Prepared For:

Division of Highways
Region One I District One
201 West Center Court
Schaumburg, Illinois 60696
www.dot.state.il.us
Prepared By:

## A=COM

303 East Wacker Drive, Suite 1400
Chicago, Illinois 60601

## TRAFFIC MANAGEMENT PLAN

```
ROUTE: IL-47
LIMITS: Reed Road to US-14
LOCATION: McHenry County
SECTION NO.: TBD
```


## Introduction

The IL-47 corridor improvement involves the reconstruction of the existing two-lane roadway into a four-lane divided highway. The improvement is approximately eight miles in length, and spans the communities of the City of Woodstock, the Village of Lake in the Hills, and the Village of Huntley. The northern terminus of the improvement is US-14. The southern terminus is Reed Road. Major high-volume crossroads include the east and west alignments of IL-176.

IL-47 is listed on the Department's Significant Route Locations Map dated 2007 at the following link. This map also appears on page 5 of this document:
http://www.dot.il.gov/illinoisshsp/WorkZoneSafetyMobility/03092009_Appendix_B.pdf
Its status as a significant route coupled with its full reconstruction scope results in a "Significant Projects-Long Term" classification and requires the preparation of this Traffic Management Plan (TMP).

The project is currently unfunded. The construction of the project is listed in the Department's current five-year plan.

## Traffic Management Plan (TMP)

The IL-47 TMP includes a Traffic Control Plan (TCP), a Transportation Operations Plan (TOP), and a Public Information Plan (PIP). The details of those plans will be finalized during Phase II engineering design, and those component plans cover the following safety and congestion mitigation strategies:

## Traffic Control Plan (TCP)

IDOT utilizes various Temporary Traffic Control Plan (TCP) strategies including signal phasing adjustments within the project limits, lane shifts, channelizing devices, temporary pavement markings, flaggers/traffic control officers, temporary signals as needed, lighting devices as needed, temporary lane closures, temporary signage, incentive/disincentive clauses in the contract documents, coordination with local stakeholders and adjacent projects, restrictions for special events as requested by the local municipalities, improvement and/or signing of alternate routes and pedestrian accommodations among others. The traffic control plans will be in conformance with State standards that will be in effect at the time of letting.

## Transportation Operations Plan (TOP)

IDOT utilizes various Transportation Operations Plan (TOP) strategies which can include traffic radio, portable changeable message signs, speed limit reduction initiatives, high occupancy vehicle (HOV) lanes, variable work hours, signal timing/coordination improvements, temporary traffic signals, alternate route improvements, parking and turn restrictions, reversible lanes, heavy vehicle restrictions, coordination with adjacent projects, incidence response coordination, Intelligent Transportation System (ITS) monitoring, surveillance through closed circuit TV (CCTV) and loop detectors, traffic screens, and local detour routes among others.

## Public Information Plan (PIP)

IDOT utilizes various Public Information Plan (PIP) strategies depending on the level of public involvement within the project, population and traveling public density, and overall resource availability within the project area. The strategies utilized can include brochures/mailers, press releases and media advisories, paid advertisements, telephone hot lines, websites, public hearings and/or meetings, press conferences, community task forces, coordination with media outlets, municipalities, schools and emergency services, work zone education campaigns and signage among others.

The following safety and congestion mitigation strategies will be implemented for the IL47 corridor improvement:

All traffic control devices will conform to the Illinois Manual on Uniform Traffic Control Devices (ILMUTCD). Temporary traffic signals will include emergency preemption and communication devices. Temporary traffic signal controllers will be supplied by one of the District approved closed loop equipment manufacturers.

Temporary traffic signals within any existing closed loop traffic signal system shall be interconnected to that system using similar brand control equipment. Traffic signal management systems shall be maintained in operation as indicated by the plans or as directed by the Resident Engineer. To best mitigate traffic queues, detection at temporary traffic signals shall be included for all approaches of the existing signalized intersections unless stated otherwise in the temporary traffic signal plans.

All signs, barricades, and temporary striping will conform to the ILMUTCD and applicable State standards. Vehicular access to local businesses and properties will be maintained at all times during construction, except when paving operations occur directly on or in front of entrances. In those cases, flag persons will be used to direct traffic. In the case of multi-entrance businesses, at least one entrance will remain open at all times. Property and business owners will be notified in advance of any temporary closures. All properties will have access at the end of every workday.

The IL-47 reconstruction will be performed in two primary stages with minor preparation of the roadway surface such as curb and gutter removal, temporary pavement installation, and temporary striping operations being performed under advanced, pre-stage work.

Stage I reconstruction will maintain existing traffic patterns of two-way traffic with one lane in each direction on the existing roadway. The existing roadway will be modified to maintain two-way traffic flow by the installation of temporary pavement in the existing median and shoulder areas. The IL-47 existing typical section accommodates one lane of travel in each direction. Stage I reconstruction will similarly maintain one lane of travel in each direction. Stage I operations will include curb and gutter removal, shoulder and pavement removal, earthwork, installation of the proposed drainage system, installation of temporary drainage pipes for maintaining drainage system connections along with the bulkheading of newly installed lateral pipes, the installation of roadway lighting and signal posts, and the reconstruction of the new pavement on the east half-section of the proposed IL-47 right-of-way. The northbound lanes will be constructed during Stage I. The suggested Stage I typical section appears on page 6.

Stage II reconstruction will establish two-way traffic with one lane in each direction on the newly reconstructed pavement established during Stage I operations. Stage II operations will include curb and gutter removal, shoulder and pavement removal, earthwork, installation of the proposed drainage system, the removal of temporary drainage pipes and lateral pipe bulkheads, the installation of roadway lighting and signal posts, and the installation of permanent landscaping and erosion control measures. The southbound lanes will be constructed during Stage II. Major operations will be completed at the end of Stage II reconstruction. The suggested Stage II typical section appears on page 6 .

The proposed right-of-way along IL-47 and its crossroads is sufficient to implement the project under the two-stage reconstruction operations summarized in the paragraphs above.

Traffic will remain open on all crossroads at all times.

Two (2) through lanes with minimum widths of 10-feet edge-to-edge (preferably 11-feet edge-to-edge) will be provided during all construction stages. The 10 -foot minimum lane widths are sufficient for emergency vehicles and truck traffic. Eleven (11) foot lanes should be used where feasible.

Similarly, the reconstruction of the structure over the Kishwaukee River will be performed in two stages that dovetail with the IL-47 staging plans.

The TMP as described in this document meets the requirements of the Work Zone Safety and Mobility Rule.

SAFETY 3-07
October 12, 2007
Page 14

## (8) IDOT DISTRICT 1 SIGNIFICANT ROUTE LOCATIONS



## Significant Locations (2007)



Consider as a Significant Route.



[^0]:    None

