State of Illinois
Department of Transportation
Bureau of Local Roads and Streets

SPECIAL PROVISION
FOR
COLD IN-PLACE RECYCLING (CIR) AND FULL-DEPTH RECLAMATION (FDR) WITH
FOAMED ASPHALT MIX DESIGN PROCEDURES

Effective: June 1, 2012

All references to Divisions, Sections, and Articles in this Special Provision shall be construed to mean specific Divisions, Sections, and Articles in the Standard Specifications for Road and Bridge Construction adopted by the Department of Transportation.

Laboratory Temperature and Humidity Control
Each laboratory performing mix designs shall have heating, ventilation, and air conditioning (HVAC) equipment that maintains a room temperature of 68 to 86 ºF (20 to 30 ºC) and relative humidity of less than 60 percent.

Sampling and Processing
A minimum sample size of 350 lb (160 kg) is needed for each mix design. Bulk samples of the recycled layer thickness shall be obtained from either test pits or cores. Each layer shall be examined to confirm thickness and material.

The bituminous layers shall be crushed. A washed gradation of the crushed bituminous layer(s) shall be performed according to AASHTO T 27, reported, and meet the following requirement(s).

Sieve Size		Percent Passing	
		CIR/FDR with Foamed Asphalt	
		Ideal	Less Suitable
2 in.	50 mm	100	
1 1/2 in.	37.5 mm	87 – 100	
1 in.	25 mm	77 – 100	100
3/4 in.	19 mm	66 – 99	99 – 100
1/2 in.	12.5 mm	67 – 87	87 – 100
3/8 in.	9.5 mm	49 – 74	74 – 100
No. 4	4.75 mm	35 – 56	56 – 95
No. 8	2.36 mm	25 – 42	42 – 78
No. 16	1.18 mm	18 – 33	33 – 65
No. 50	300 µm	10 – 24	24 – 43
No. 200	75 µm	4 – 10	10 – 20

Washed gradation (AASHTO T 27) and sand equivalent (ASTM D 2419, Method B) shall be performed and reported for any granular layer. The washed gradation (AASHTO T 27) of combined layers shall be performed and reported. If combined layers include an aggregate layer, the sand equivalent (ASTM D 2419, Method B) shall be performed and reported.

All washed gradations shall be dried at no greater than 104 °F (40 °C).

Active filler requirements
Foamed asphalt stabilization is normally carried out in combination with a small amount of active filler (cement, fly ash, or lime) to enhance the dispersion of the foamed asphalt. The following application rates (by mass) of cement, fly ash, or lime should be used as a guide:

Plasticity Index: < 10	Plasticity Index: > 10
Add 1 percent ordinary portland cement or 1 percent lime (material dependent)	Pre-treat with minimum 2 percent lime. The initial consumption of lime (ICL) has to be satisfied.

Pre-treatment requires that the lime and water be added at least four hours prior to the addition of the foamed asphalt. The treated material must be placed in an air-tight container to retain moisture. However, due to the hydration process, the moisture content should always be checked and, if necessary, adjusted prior to adding the foamed asphalt.

Note: Additional tests without active filler should always be carried out as part of the mix design process. The results of these tests allow a decision to be made as to whether the addition of an active filler is warranted.

Mixing and Compaction
The Optimum Fluid Content (OFC) and the Maximum Dry Density (MDD) of the stabilized material is determined using modified compaction effort (Modified Proctor, ASTM D 1557, Method C).

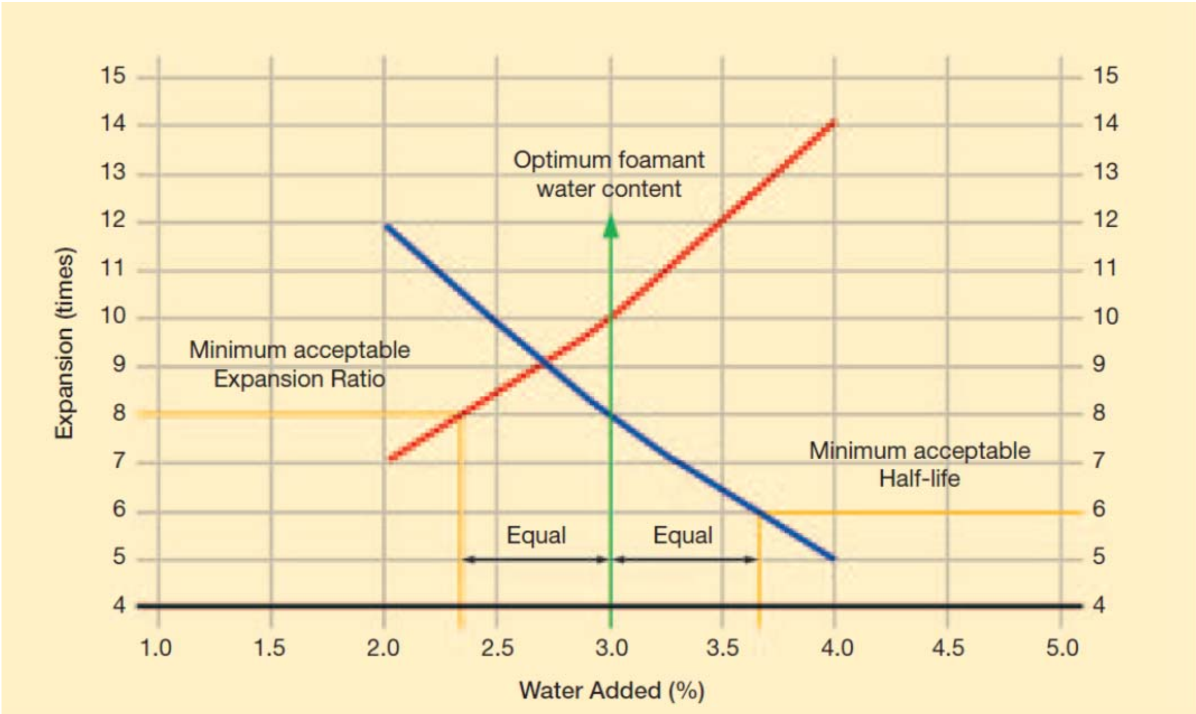
Determination of Expansion Ratio and Half-Life
The foaming properties of asphalt are characterized by:

- Expansion Ratio. A measure of the viscosity of the foamed bitumen, calculated as the ratio of the maximum volume of the foam relative to the original volume of bitumen.
- Half-Life. A measure of the stability of the foamed bitumen, calculated as the time taken in seconds for the foam to collapse to one-half of its maximum volume.

The objective is to determine the temperature and percentage of water addition that is required to produce the best foam properties (maximum expansion ratio and half-life) for a particular source of bitumen. This is achieved at three different bitumen temperatures not exceeding 380 °F (195 °C) with the following procedure.

1. Heat the bitumen in the kettle foaming laboratory unit with the pump circulating the bitumen through the system until the required temperature is achieved normally starting with 320 °F (160 °C). Maintain the required temperature for at least five minutes prior to commencing with testing.
2. Calibrate the discharge rate of the bitumen and set the timer on the foaming laboratory unit to discharge 500 g of bitumen ($Q_{bitumen}$).
3. Set the water flow-meter to achieve the required water injection rate normally starting with 2 percent by mass of the bitumen.
4. Discharge foamed bitumen into steel drum preheated to ± 135 °F (± 75 °C) of the bitumen for a calculated spray time for 500 g of bitumen. Immediately after the foam discharge stops, start a stopwatch.
5. Using the calibrated dipstick supplied with the foaming laboratory unit measure the maximum height the foamed bitumen achieves in the drum. This is recorded as the maximum volume.

6. Use the stopwatch to measure the time in seconds that the foam takes to dissipate to one-half of its maximum volume. This is recorded as the foamed bitumen's half-life.
7. Repeat the above procedures three times or until similar readings are achieved.
8. Repeat Steps 3 through 7 for a range of at least three water injection rates. Typically, values of 2 percent, 3 percent and 4 percent by mass of bitumen are used.
9. Plot a graph of the expansion ratio versus half-life at the different water injection rates on the same set of axes (see an example in the graph below). The optimum water addition is chosen as an average of the two water contents required to meet these minimum criteria.



Repeat Steps 1 through 9 for two other bitumen temperatures normally 340 °F (170 °C) and 360 °F (180 °C). The temperature and optimum water addition that produces the best foam is then used in the mix design procedure described below.

Sample preparation for foamed bitumen treatment
Prepare the material for foamed bitumen treatment as follows:

1. Place 20 to 25 kg of prepared sample into the pug mill mixer.
2. Determine the dry mass of the sample using the following equation:

$$m_{sample} = \frac{m_{air-dry}}{\left(1 + \left(\frac{w_{air-dry}}{100}\right)\right)}$$

Where: m_{sample} = dry mass of the sample in grams
$m_{air\text{-}dry}$ = air-dried mass of the sample in grams
$w_{air\text{-}dry}$ = moisture content of air-dried sample in percent by mass

3. Determine the required percentage of active filler (lime, cement, or fly ash) using the following equation:

$$m_{cement} = \left(\frac{w_{c-add}}{100}\right) m_{sample}$$

Where: m_{cement} = mass of lime, cement, or fly ash to be added in grams
$w_{c\text{-}add}$ = percentage of lime, cement, or fly ash required in percent by mass
m_{sample} = dry mass of the sample in grams

4. Determine the percentage of water to be added for optimum mixing moisture and the amount of water to be added to the sample using the following equations:

$$W_{add} = 0.75W_{OMC} - W_{air-dry}$$

$$m_{water} = \left(\frac{w_{add}}{100}\right)\left(m_{sample} + m_{cement}\right)$$

where: w_{add} = water to be added to sample in percent by mass
w_{OMC} = optimum moisture content in percent by mass
$w_{air\text{-}dry}$ = moisture content of air-dried sample in percent by mass
m_{water} = mass of water to be added in grams
m_{sample} = dry mass of the sample in grams
m_{cement} = mass of lime, cement or fly ash to be added in grams

5. Mix the material, active filler, and water in the mixer until uniform.

Note: Inspect the sample after mixing to ensure that the mixed material is not packed against the sides of the mixer. If this situation occurs, mix a new sample at a lower moisture content. Check to see that the material mixes easily and remains in a "fluffed" state. If any dust is observed at the end of the mixing process, add small amounts of water and remix until a "fluffed" state is achieved with no dust.

6. Determine the amount of foamed bitumen to be added using the following equation:

$$m_{bitumen} = \left(\frac{w_{b-add}}{100}\right)\left(m_{sample} + m_{cement}\right)$$

where: $m_{bitumen}$ = mass of foamed bitumen to be added in grams
$w_{b\text{-}add}$ = foamed bitumen content in percent by mass
m_{sample} = dry mass of the sample in grams
m_{cement} = mass of lime, cement or fly ash to be added in grams

7. Determine the timer setting on the foaming laboratory unit using the following equation:

$$t = \frac{m_{bitumen}}{Q_{bitumen}}$$

where: t = time to be set on the foaming laboratory unit timer
$m_{bitumen}$ = mass of foamed bitumen to be added in grams
$Q_{bitumen}$ = bitumen flow rate for the foaming laboratory unit in grams/second

8. Position the mixer adjacent to the foaming unit so that the foamed bitumen can be discharged directly into the mixing chamber.

9. Start the mixer and allow it to mix for at least 10 seconds before discharging the required mass of foamed bitumen into the mixing chamber. After the foamed bitumen has discharged into the mixer, continue mixing for an additional 30 seconds or until uniformly mixed.

10. The moisture content of the material is to be adjusted to 90 percent of optimum moisture content.

11. Add the additional water and mix until uniform.

12. Transfer the foamed bitumen treated material into a container and immediately seal the container to retain moisture. To minimize moisture loss from the prepared sample, compact the specimens as soon as possible.

Repeat the above steps for at least four different foamed asphalt contents.

Compaction

Six specimens are manufactured for each sample at the different bitumen contents. Compact the specimens as follows:

1. Prepare the Marshall mold and hammer by cleaning the mold, collar, base-plate and face of the compaction hammer.

   Note: The compaction equipment must not be heated but kept at ambient temperature.

2. Weigh sufficient material to achieve a compacted height of 2.5 ± 0.125 in. (63.5 ± 1.5 mm) (usually 1150 g is adequate). Poke the mixture with a spatula 15 times around the perimeter and 10 times on the surface, leaving the surface slightly rounded.

3. Compact the mixture by applying 75 blows with the compaction hammer. Care must be taken to ensure the continuous free fall of the hammer.

4. Take ± 1000 g representative samples after compaction of the second and fifth specimen and dry to a constant mass at 220 to 230 °F (105 to 110 °C). Determine the molding moisture using the following equation:

   $$w_{mold} = \left(\frac{m_{moist} - m_{dry}}{m_{dry}}\right)100$$

   where: w_{mold} = molding moisture content in percent by mass
   m_{moist} = mass of moist material in grams
   m_{dry} = mass of dry material in grams

5. Remove the mold and collar from the pedestal, invert the specimen (turn over). Replace it and press down firmly to ensure that it is secure on the base plate. Compact the other face of the specimen with an additional 75 blows.

6. After compaction, remove the mold from the base-plate and extrude the specimen by means of an extrusion jack. Measure the height of the specimen and adjust the amount material if the height is not within the required limits.

   Note: With certain materials lacking cohesion, it may be necessary to leave the specimen in the mold for 24 hours, allowing sufficient strength to develop before extracting.

Curing after Compaction

Specimens shall be cured for 72 hours at 104 °F (40 °C). The bottom of the specimens shall rest on racks with slots or holes for air circulation. After curing, specimens for moisture conditioning shall be cooled at ambient temperature a maximum of 24 hours; specimens for dry strength shall cool at ambient temperature or 77 °F (25 °C) and be tested at the same time as moisture-conditioned specimens.

Specimens for Rice (maximum theoretical) specific gravity shall be cured at the same conditions as the compacted specimens, except they can be tested after cooling a maximum of 24 hours.

Volumetric Measurements

Determine bulk specific gravity (ASTM D 6752) of the specimens. Keep specimens in bags until testing or vacuum saturation is performed. ASTM D 2726 may be used to determine bulk specific gravity if specimens' absorption is less than or equal to 2 percent of water by volume.

Determine Rice (maximum theoretical) specific gravity (ASTM D 2041).

Determine air voids at all foamed asphalt contents used in the design.

Mechanical Measurements
Perform ITS testing according to AASHTO T 283 (IL Modified). Specimens shall be conditioned at 77 °F (25 °C) for two hours before testing. Vacuum saturate one-half of the specimens at each foamed asphalt content to a minimum 55 percent of the voids filled with water. Soak for 24 hours at 77 °F (25 °C) before testing.

Raveling Test (CIR with Foamed Asphalt Only)
The apparatus used for the raveling test is a modified A-120 Hobart mixer and abrasion head (including hose) used in the Wet Track Abrasion of Slurry Surfaces Test (ISSA TB-100). The rotation speed for the raveling test is not modified from ISSA TB-100. The ring weight is removed from the abrasion head for the raveling test below. The weight of the abrasion head and hose in contact with the specimen should be 600 g ± 15 g. The prepared sample must be able to be secured under the abrasion head, and centered for an accurate result, allowing for free movement vertically of the abrasion head. The device used for securing and centering the sample must allow a minimum of 0.4 in. (10 mm) of the sample to be available for abrasion. The Hobart mixer will need to be modified to allow the sample to fit properly for abrasion. The modification may be accomplished by adjusting the abrasion head height, or the height of the secured sample. The Hobart C-100 and N-50 Models are not acceptable for this test procedure due to differences in size and speed of rotation.

1. Split out two recycled asphalt samples from the medium gradation, or field sample, to a quantity of 2700 g in mass. The 2700 g is an approximate weight to give 2.8 in. ± 0.2 in. (70 mm ± 5 mm) of height after compaction.
2. The recycled asphalt sample should be placed in a container of adequate size for mixing.
3. Field or design moisture contents should be added to each of the recycled asphalt samples and mixed for 60 seconds.
4. The design emulsion content shall be added to each of the recycled asphalt samples and mixed for 60 seconds.
5. The samples shall be placed immediately into a 6 in. (150 mm) gyratory compaction mold and compacted to 20 gyrations. If the sample height is not 2.8 in. ± 0.2 in. (70 mm ± 5 mm), the recycled asphalt weight should be adjusted.
6. After compaction, the samples shall be removed from the compaction mold and placed on a flat pan to cure at the specified temperature and humidity (if required) for 240 minutes ± 5 minutes. The temperature shall be maintained at 50 ºF ± 3.5 ºF (10 ºC ± 2 ºC).
7. The specimens shall be weighed after the curing, just prior to testing.
8. The specimens shall be placed on the raveling test apparatus. Care should be taken that the specimen is centered and well supported. The area of the hose in contact with the specimen should not have been previously used. It is allowable to rotate the hose to an unworn section for testing. The abrasion head (with hose) shall be free to move vertically downward a minimum of 0.2 in. (5 mm) if abrasion allows.
9. The samples shall be abraded for 15 minutes and immediately weighed.
10. The Percent Raveling Loss shall be determined as follows:

$$PRL = 100 \times \frac{W_P - W_A}{W_p}$$

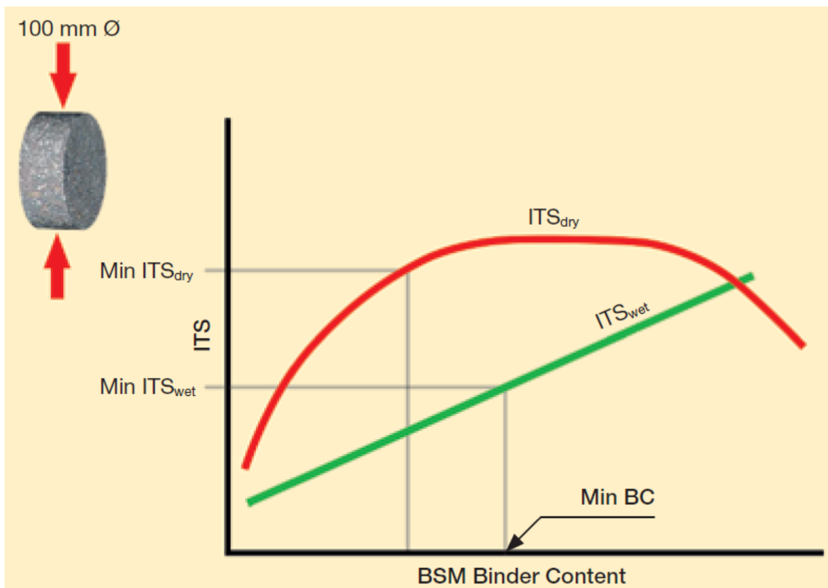
Where: PRL = Percent Raveling Loss
W_P = Weight of Sample Prior to Testing
W_A = Weight of Sample After Testing

11. The average of the two specimens shall be reported as the Percent Raveling Loss. If there is a difference of > 0.5 percent raveling loss between the two test specimens, the Raveling Test shall be repeated. If both of the test specimens have a Percent Raveling Loss of > 10 percent, the two test results shall be averaged and the maximum 0.5 percent difference between test specimens shall not be required.

Note: If field mix samples are taken, steps 2, 3, and 4 shall be omitted.

Foamed Asphalt Content Selection

The results of the respective soaked and unsoaked ITS test results are plotted against the relevant bitumen content that was added. The added bitumen content that best meets the desired Bitumen Stabilized Material (BSM) classification is selected as the amount of bitumen to be added, as shown in the example below.



Report

All mix design test results shall be reported to the Department. All additional additives and bituminous material shall be reported to the Department.