



# Illinois Department of Transportation

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To: John Fortman Attn: District One  
From: John D. Baranzelli  
Subject: Pavement Design  
Date: September 4, 2012

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A handwritten signature in black ink, appearing to be 'JDB'.

Ramps G5, G6 and K4  
Section 03-96-0021  
Du Page County  
At Elgin-O'Hare Expressway

We have reviewed the pavement selection for the project, which was submitted to BDE by email dated August 20, 2012. The project will reconstruct these ramps at the Elgin-O'Hare Expressway. The life cycle cost analysis for ramp G5 favored the rigid design by 19%. The approved pavement design for this project is as follows:

Ramps G5, G6 and K4 [Pavement Reconstruction]

10 inches of PCC Jointed Pavement with Tied PCC Shoulders  
4.5 HMA Stabilized Subbase  
12 inches of Aggregate Subgrade Improvement

If you have any questions, please contact Paul Niedernhofer at (217) 524-1651.



# Illinois Department of Transportation

## Memorandum

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To: John D. Baranzelli                      Attn: Paul Niedernhofer  
From: John Fortmann                      By: Jose Dominguez  
Subject: Pavement Analysis\*  
Date: August 20, 2012

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\*Route: Ramps G5, G6, and K4  
Limits: at Elgin-O'Hare Expressway  
Contract No.: 11838  
Letting: By ISTHA

Section: 03-96-0021  
County: Cook  
Job No.: P-91-443-06

We have completed the pavement analysis for the above captioned locations. Review by the Central Office is required for Ramp G5 since the total pavement area for new construction exceeds 4,750 Square Yards. The following is the scope of the project:

- a.) New construction of Ramp G5 at the Elgin-O'Hare for approximately 5,157 feet.
- b.) New construction of Ramp G6 at the Elgin-O'Hare for approximately 2,310 feet.
- c.) New construction of Ramp K4 at the Elgin-O'Hare for approximately 2,181 feet.

A 20 year pavement analysis was performed on Ramp G5 since the pavement new construction is less than 25,000 square yards. We recommend a mechanistic-rigid pavement design based on the life cycle cost analysis which favors PCC pavement by 19%.

A segmental pavement analysis was performed on both Ramps G6 and K4 and it is recommended to match Ramp G5 and use a rigid pavement design. Our recommendation is as follows.

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**a.) Ramp G5, G6 K4**

Pavement New Construction

Tied PCC Shoulders

10" PCC Pavement Jointed <sup>1</sup>

4 ½" HMA Stabilized Subbase <sup>2</sup>

12" Aggregate Subgrade Improvement <sup>3</sup>

John D. Baranzelli  
August 20, 2012  
Page Two

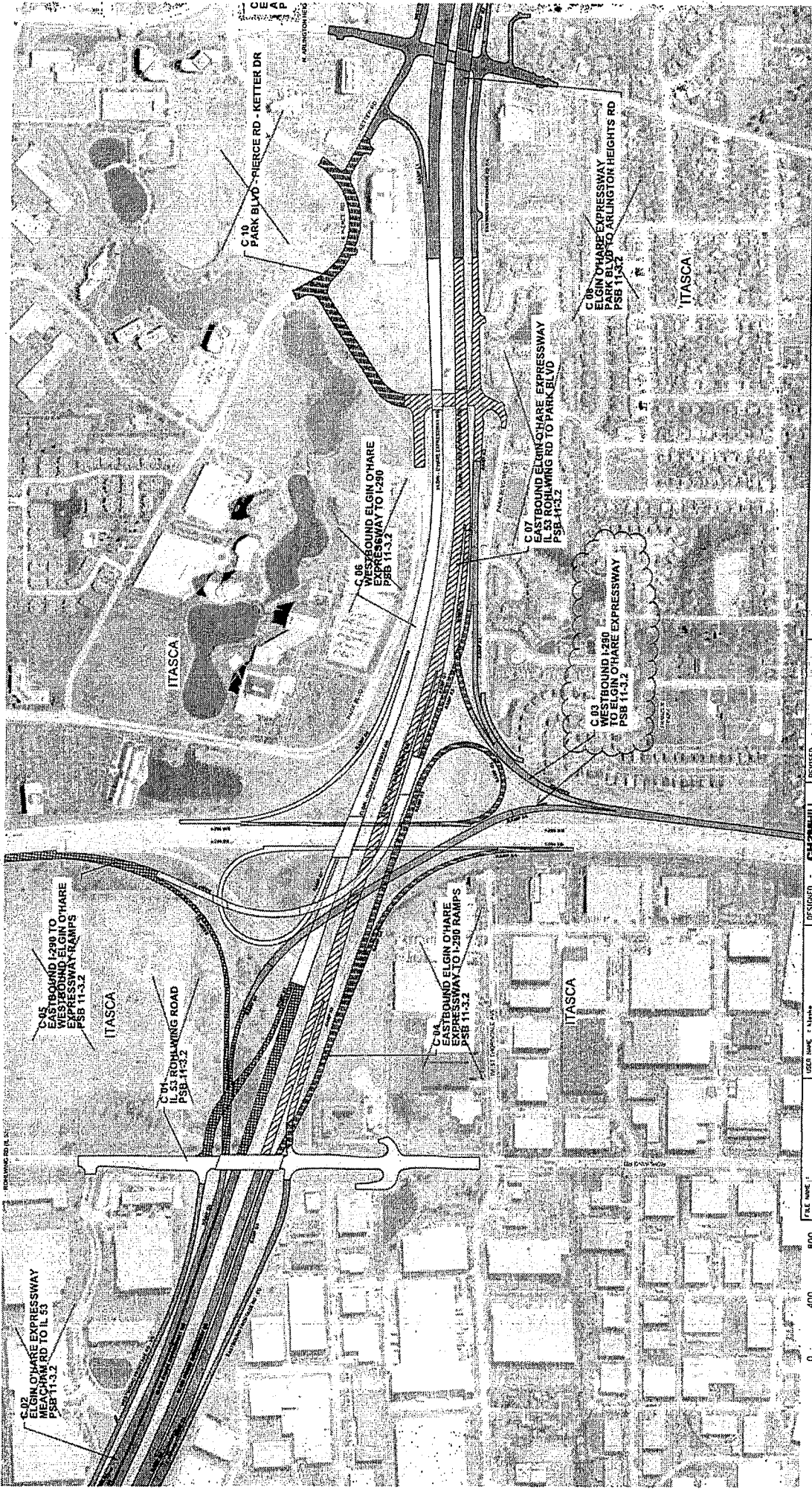
<sup>1</sup> Designer Note 1: Use pay item #42000501, "PORTLAND CEMENT CONCRETE PAVEMENT, 10" (JOINTED)", paid in square yards.

<sup>2</sup> Designer Note 2: Use pay item #31200502, "STABILIZED SUBBASE - HOT-MIX ASPHALT, 4 ½" ", paid in square yards.

<sup>3</sup> Designer Note 3: Use pay item #30300112, "AGGREGATE SUBGRADE IMPROVEMENT, 12" ", paid in square yards.

If you have any questions or need additional information, please contact Jenpai Chang, Acting Pavement Design Engineer, at (847)705-4432.

By: *Jose A. Dominguez*  
Jose A. Dominguez, P.E.  
Project Support Engineer



C-02  
ELGIN O'HARE EXPRESSWAY  
PARK BLVD TO I-55  
PSB 11-3.2

C-05  
EASTBOUND I-290 TO  
WESTBOUND ELGIN O'HARE  
EXPRESSWAY RAMP  
PSB 11-3.2

C-04  
ROHLFING ROAD  
PSB 11-3.2

C-08  
EASTBOUND ELGIN O'HARE  
EXPRESSWAY TO I-290 RAMP  
PSB 11-3.2

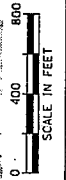
C-06  
WESTBOUND ELGIN O'HARE  
EXPRESSWAY TO I-290  
PSB 11-3.2

C-07  
EASTBOUND ELGIN O'HARE EXPRESSWAY  
I-55 ROHLFING RD TO PARK BLVD  
PSB 11-3.2

C-09  
BOUND I-290  
TO ELGIN O'HARE EXPRESSWAY  
PSB 11-3.2

C-10  
ELGIN O'HARE EXPRESSWAY  
PARK BLVD TO ARLINGTON HEIGHTS RD  
PSB 11-3.2

C-10  
PARK BLVD - PIERCE RD - KETTER DR



FILE NO. 11-3.2  
DESIGNED BY: CH2M HILL  
CHECKED BY: CH2M HILL  
DATE: 11/13/02

DESIGNED BY: CH2M HILL  
CHECKED BY: CH2M HILL  
DATE: 11/13/02

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CHECKED BY: CH2M HILL  
DATE: 11/13/02

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY  
2700 OGDEN AVENUE  
DOWNERS GROVE, ILLINOIS 60515

CONTRACT PACKAGING  
ELGIN O'HARE EXPRESSWAY

SCALE: SHEET NO. OF SHEETS: STA. TO STA.


SCALE: SHEET NO. OF SHEETS: STA. TO STA.

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PROJECT AND TRAFFIC INPUTS				(Enter Data in Gray Shaded Cells)			
Route: Elgin O'Hare-West Bypass	Comments: New Construction						
Section:	Design Date: 08/09/2012	MR	← BY				<b>READ ME</b>
County: Dupage	Modified Date:		← BY	ADT	Year		
Location: Ramp G-5			Current:	17,900	2010		
Facility Type: Other Marked State Route	** Ramp Design Fig. 54-1.B **		Future:	16,100	2030		
# of Lanes = 1 Lane Ramp	Crossroad? Interstate or Supplemental Freeway	# of Lanes = 4	Structural Design Traffic				
	Road Class: I	Road Class: I	Minimum ADT	Actual ADT	Actual % of Total ADT	% of ADT in Design Lane	
	Subgrade Support Rating (SSR): Poor		PV = 0	15,542	92.9%	P = 100%	
	Construction Year: 2013		SU = 500	535	3.2%	S = 100%	
	Design Period (DP) = 20 years		MU = 1500	652	3.9%	M = 100%	
			Struct. Design ADT = 16,730	(2023)			

TRAFFIC FACTOR CALCULATION							
FLEXIBLE PAVEMENT				RAMP DESIGN MIN.			
Cpv = 0.15		0.15	32%	Cpv = 0.15		0.15	32%
Csu = 132.5		132.5	45%	Csu = 143.81		143.81	45%
Cmu = 482.53		482.53	45%	Cmu = 696.42		696.42	45%
TF flexible (Actual) = 7.76		(Actual ADT) 7.11		TF rigid (Actual) = 10.67		(Actual ADT) 10.05	
TF flexible (Min) = 7.11		(Min ADT Fig. 54-2.C)		TF rigid (Min) = 10.05		(Min ADT Fig. 54-2.C)	

NEW CONSTRUCTION / RECONSTRUCTION PAVEMENT DESIGN CALCULATIONS							
Full-Depth HMA Pavement				JPC Pavement			
Use TF flexible = 7.76	PG Grade Lower Binder Lifts = PG 64-22 (Fig. 53-4.R)			Use TF rigid = 10.67	Edge Support = Tied Shoulder or C.&G.		
HMA Mixture Temp. = 74.0 deg. F (Fig. 54-5.C)	Design HMA Mixture Modulus (E <sub>HMA</sub> ) = 720 ksi (Fig. 54-5.D)			Rigid Pavt Thick. = 10.00 in. (Fig. 54-4.E)			
Design HMA Strain (ε <sub>HMA</sub> ) = 67 (Fig. 54-5.E)	Full Depth HMA Design Thickness = 11.50 in. (Fig. 54-5.F)			CRC Pavement			
Limiting Strain Criterion Thickness = 14.50 in. (Fig. 54-5.I)	Use Full-Depth HMA Thickness = 11.50 inches			Use TF rigid = 10.67	IBR value = 3		
				CRCP Thickness = 9.00 in. (Fig. 54-4.M)			
<b>TF MUST BE &gt; 60 FOR CRCP</b>							

RECONSTRUCTION ONLY (SUPPLEMENTAL) PAVEMENT DESIGN CALCULATIONS							
HMA Overlay of Rubblized PCC				Unbonded Concrete Overlay			
Use TF flexible = 7.76	District = 3,4,5,6			Review 54-4.03 for limitations and special considerations.			
HMA Overlay Design Thickness = 9.75 in. (Fig. 54-5.U)				JPCP Thickness = NA inches			
<b>CONTACT BMPR FOR ASSISTANCE</b>							

DESIGN TABLES FROM BDE MANUAL CHAPTER 54 - PAVEMENT DESIGN							
Class I Roads		Class II Roads		Class III Roads		Class IV Roads	
4 lanes or more Part of a future 4 lanes or more One-way Streets with ADT > 3500		2 lanes with ADT > 2000 One way Street with ADT <= 3500		2 Lanes (ADT 750 -2000)		2 Lanes (ADT < 750)	
		Min. Str. Design Traffic (Fig 54-2.C)					
Facility Type		PV	SU*	MU*		Class Table for One-Way Streets	
Interstate or Supplemental Freeway		0	500	1500		ADT	Class
Other Marked State Route		0	250	750		0 - 3500	II
Unmarked State Route		0	250	750		>3501	I
* Use marked route minimums for unmarked routes (Fig. 54-1.B)							
		Traffic Factor ESAL Coefficients					
		Rigid (Fig. 54-4.C)		Flexible (Fig. 54-5.B)			
Class	Csu	Cmu	Csu	Cmu			
I	143.81	696.42	132.50	482.53			
II	135.78	567.21	112.06	385.44			
III	129.58	562.47	109.14	384.35			
IV	129.58	562.47	109.14	384.35			
		Design Lane Distribution Factors For Structural Design Traffic (Fig. 54-2.B)					
		Rural			Urban		
Number of Lanes	P	S	M	P	S	M	
1 Lane Ramp	100%	100%	100%	100%	100%	100%	
2 or 3	50%	50%	50%	50%	50%	50%	
4	32%	45%	45%	32%	45%	45%	
6 or more	20%	40%	40%	8%	37%	37%	
						Class Table for 2 or 3 lanes (not future 4 lane & not one-way street)	
						ADT	Class
						0 - 749	IV
						750 - 2000	III
						>2000	II

**PROJECT AND TRAFFIC INPUTS** (Enter Data in Gray Shaded Cells)

Route: Elgin O'Hare-West Bypass		Comments: New Construction																							
Section:		Design Date: 08/09/2012		MR																					
County: Dupage		Modified Date:		<table border="1" style="display: inline-table; border-collapse: collapse;"> <tr> <td>&lt;- BY</td> <td>ADT</td> <td>Year</td> </tr> <tr> <td>&lt;- BY</td> <td>10,000</td> <td>2010</td> </tr> <tr> <td>Current:</td> <td>18,400</td> <td>2030</td> </tr> <tr> <td>Future:</td> <td></td> <td></td> </tr> </table>		<- BY	ADT	Year	<- BY	10,000	2010	Current:	18,400	2030	Future:										
<- BY	ADT	Year																							
<- BY	10,000	2010																							
Current:	18,400	2030																							
Future:																									
Location: Ramp G-6				<b>READ ME</b>																					
Facility Type: Other Marked State Route		** Ramp Design Fig. 54-1.B **																							
# of Lanes = 1 Lane Ramp		Crossroad? Interstate or Supplemental Freeway																							
		# of Lanes = 4		Structural Design Traffic																					
Road Class: I		Road Class: I		<table border="1" style="display: inline-table; border-collapse: collapse;"> <tr> <td>Minimum ADT</td> <td>Actual ADT</td> <td>Actual % of Total ADT</td> <td>% of ADT in Design Lane</td> </tr> <tr> <td>PV = 0</td> <td>14,022</td> <td>90.7%</td> <td>P = 100%</td> </tr> <tr> <td>SU = 500</td> <td>758</td> <td>4.9%</td> <td>S = 100%</td> </tr> <tr> <td>MU = 1500</td> <td>680</td> <td>4.4%</td> <td>M = 100%</td> </tr> <tr> <td colspan="2">Struct. Design ADT = 15,460</td> <td colspan="2">(2023)</td> </tr> </table>		Minimum ADT	Actual ADT	Actual % of Total ADT	% of ADT in Design Lane	PV = 0	14,022	90.7%	P = 100%	SU = 500	758	4.9%	S = 100%	MU = 1500	680	4.4%	M = 100%	Struct. Design ADT = 15,460		(2023)	
Minimum ADT	Actual ADT	Actual % of Total ADT	% of ADT in Design Lane																						
PV = 0	14,022	90.7%	P = 100%																						
SU = 500	758	4.9%	S = 100%																						
MU = 1500	680	4.4%	M = 100%																						
Struct. Design ADT = 15,460		(2023)																							
Subgrade Support Rating (SSR): Poor		Construction Year: 2013																							
Design Period (DP) = 20 years																									

**TRAFFIC FACTOR CALCULATION**

FLEXIBLE PAVEMENT		RAMP DESIGN MIN		RIGID PAVEMENT		RAMP DESIGN MIN	
Cpv =	0.15	0.15	32%	Cpv =	0.15	0.15	32%
Csu =	132.5	132.5	45%	Csu =	143.81	143.81	45%
Cmu =	482.53	482.53	45%	Cmu =	696.42	696.42	45%
TF flexible (Actual) =	8.61	(Actual ADT)	7.11	TF rigid (Actual) =	11.70	(Actual ADT)	10.05
TF flexible (Min) =	7.11	(Min ADT Fig. 54-2.C)		TF rigid (Min) =	10.05	(Min ADT Fig. 54-2.C)	

**NEW CONSTRUCTION / RECONSTRUCTION PAVEMENT DESIGN CALCULATIONS**

Full-Depth HMA Pavement	JPC Pavement
Use TF flexible = 8.61	Use TF rigid = 11.70
PG Grade Lower Binder Lifts = PG 64-22 (Fig. 53-4.R)	Edge Support = Tied Shoulder or C.&G.
HMA Mixture Temp. = 74.0 deg. F (Fig. 54-5.C)	Rigid Pavt Thick. = 10.00 in. (Fig. 54-4.E)
Design HMA Mixture Modulus (E <sub>HMA</sub> ) = 720 ksi (Fig. 54-5.D)	
Design HMA Strain (ε <sub>HMA</sub> ) = 65 (Fig. 54-5.E)	<b>CRCP Pavement</b>
Full Depth HMA Design Thickness = 11.75 in. (Fig. 54-5.F)	Use TF rigid = 11.70
Limiting Strain Criterion Thickness = 14.50 in. (Fig. 54-5.I)	IBR value = 3
Use Full-Depth HMA Thickness = 11.75 inches	CRCP Thickness = 9.00 in. (Fig. 54-4.M)
	<b>TF MUST BE &gt; 60 FOR CRCP</b>

**RECONSTRUCTION ONLY (SUPPLEMENTAL) PAVEMENT DESIGN CALCULATIONS**

HMA Overlay of Rubblized PCC	Unbonded Concrete Overlay
Use TF flexible = 8.61	Review 54-4.03 for limitations and special considerations.
District = 3,4,5,6	JPCP Thickness = NA inches
HMA Overlay Design Thickness = 10.00 in. (Fig. 54-5.U)	

**CONTACT BMPR FOR ASSISTANCE**

**DESIGN TABLES FROM BDE MANUAL CHAPTER 54 - PAVEMENT DESIGN**

Class I Roads	Class II Roads	Class III Roads	Class IV Roads
4 lanes or more	2 lanes with ADT > 2000	2 Lanes	2 Lanes
Part of a future 4 lanes or more	One way Street with ADT <= 3500	(ADT 750 -2000)	(ADT < 750)
One-way Streets with ADT > 3500			

Facility Type	Min. Str. Design Traffic (Fig 54-2.C)		
	PV	SU*	MU*
Interstate or Supplemental Freeway	0	500	1500
Other Marked State Route	0	250	750
Unmarked State Route	0	250	750

\* Use marked route minimums for unmarked routes (Fig. 54-1.B)

Class Table for One-Way Streets	
ADT	Class
0 - 3500	II
>3501	I

Class	Traffic Factor ESAL Coefficients			
	Rigid (Fig. 54-4.C)		Flexible (Fig. 54-5.B)	
	Csu	Cmu	Csu	Cmu
I	143.81	696.42	132.50	482.53
II	135.78	567.21	112.06	385.44
III	129.58	562.47	109.14	384.35
IV	129.58	562.47	109.14	384.35

Class Table for 2 or 3 lanes (not future 4 lane & not one-way street)	
ADT	Class
0 - 749	IV
750 - 2000	III
>2000	II

Number of Lanes	Design Lane Distribution Factors For Structural Design Traffic (Fig. 54-2.B)					
	Rural			Urban		
	P	S	M	P	S	M
1 Lane Ramp	100%	100%	100%	100%	100%	100%
2 or 3	50%	50%	50%	50%	50%	50%
4	32%	45%	45%	32%	45%	45%
6 or more	20%	40%	40%	8%	37%	37%

**PROJECT AND TRAFFIC INPUTS** (Enter Data in Gray Shaded Cells)

Route: Elgin O'Hare-West Bypass		Comments: New Construction							
Section:		Design Date: 08/09/2012	MR						
County: Dupage		Modified Date:							
Location: Ramp K-4									
Facility Type: Other Marked State Route	** Ramp Design Fig. 54-1.B **	Current: 3,500	Year: 2010						
		Future: 6,000	2030						
# of Lanes = 1 Lane Ramp	Crossroad? Interstate or Supplemental Freeway	<table border="1" style="float: right;"> <tr><th>ADT</th><th>Year</th></tr> <tr><td>3,500</td><td>2010</td></tr> <tr><td>6,000</td><td>2030</td></tr> </table>		ADT	Year	3,500	2010	6,000	2030
ADT	Year								
3,500	2010								
6,000	2030								
	# of Lanes = 4								
Road Class: I	Road Class: I	Structural Design Traffic							
Subgrade Support Rating (SSR): Poor	Construction Year: 2013	Minimum ADT	Actual ADT						
Design Period (DP) = 20 years		Actual % of Total ADT	% of ADT in Design Lane						
		PV = 0	P = 100%						
		SU = 500	S = 100%						
		MU = 1500	M = 100%						
		Struct. Design ADT = 5,125	(2023)						

RESET

READ ME

**TRAFFIC FACTOR CALCULATION**

FLEXIBLE PAVEMENT		RAMP DESIGN MIN		RIGID PAVEMENT		RAMP DESIGN MIN	
Cpv =	0.15	0.15	32%	Cpv =	0.15	0.15	32%
Csu =	132.5	132.5	45%	Csu =	143.81	143.81	45%
Cmu =	482.53	482.53	45%	Cmu =	696.42	696.42	45%
TF flexible (Actual) =	0.56	(Actual ADT)	7.11	TF rigid (Actual) =	0.76	(Actual ADT)	10.05
TF flexible (Min) =	7.11	(Min ADT Fig. 54-2.C)		TF rigid (Min) =	10.05	(Min ADT Fig. 54-2.C)	

**NEW CONSTRUCTION / RECONSTRUCTION PAVEMENT DESIGN CALCULATIONS**

Full-Depth HMA Pavement	JPC Pavement
Use TF flexible = 7.11	Use TF rigid = 10.05
PG Grade Lower Binder Lifts = PG 64-22 (Fig. 53-4.R)	Edge Support = Tied Shoulder or C.&G.
HMA Mixture Temp. = 74.0 deg. F (Fig. 54-5.C)	Rigid Pavt Thick. = 10.00 in. (Fig. 54-4.E)
Design HMA Mixture Modulus (E <sub>HMA</sub> ) = 720 ksi (Fig. 54-5.D)	
Design HMA Strain (ε <sub>HMA</sub> ) = 69 (Fig. 54-5.E)	
Full Depth HMA Design Thickness = 11.25 in. (Fig. 54-5.F)	
Limiting Strain Criterion Thickness = 14.50 in. (Fig. 54-5.I)	
Use Full-Depth HMA Thickness = 11.25 inches	CRCP Thickness = 9.00 in. (Fig. 54-4.M)

**TF MUST BE > 60 FOR CRCP**

**RECONSTRUCTION ONLY (SUPPLEMENTAL) PAVEMENT DESIGN CALCULATIONS**

HMA Overlay of Rubblized PCC	Unbonded Concrete Overlay
Use TF flexible = 7.11	Review 54-4.03 for limitations and special considerations.
District = 3,4,5,6	JPCP Thickness = NA inches
HMA Overlay Design Thickness = 9.50 in. (Fig. 54-5.U)	

**CONTACT BMPR FOR ASSISTANCE**

**DESIGN TABLES FROM BDE MANUAL CHAPTER 54 - PAVEMENT DESIGN**

Class I Roads	Class II Roads	Class III Roads	Class IV Roads
4 lanes or more Part of a future 4 lanes or more One-way Streets with ADT > 3500	2 lanes with ADT > 2000 One way Street with ADT <= 3500	2 Lanes (ADT 750 -2000)	2 Lanes (ADT < 750)

Facility Type	Min. Str. Design Traffic (Fig 54-2.C)		
	PV	SU*	MU*
Interstate or Supplemental Freeway	0	500	1500
Other Marked State Route	0	250	750
Unmarked State Route	0	250	750

\* Use marked route minimums for unmarked routes (Fig. 54-1.B)

Class Table for One-Way Streets	
ADT	Class
0 - 3500	II
>3501	I

Class	Traffic Factor ESAL Coefficients			
	Rigid (Fig. 54-4.C)		Flexible (Fig. 54-5.B)	
	Csu	Cmu	Csu	Cmu
I	143.81	696.42	132.50	482.53
II	135.78	567.21	112.06	385.44
III	129.58	562.47	109.14	384.35
IV	129.58	562.47	109.14	384.35

Class Table for 2 or 3 lanes (not future 4 lane & not one-way street)	
ADT	Class
0 - 749	IV
750 - 2000	III
>2000	II

Number of Lanes	Design Lane Distribution Factors For Structural Design Traffic (Fig. 54-2.B)					
	Rural			Urban		
	P	S	M	P	S	M
1 Lane Ramp	100%	100%	100%	100%	100%	100%
2 or 3	50%	50%	50%	50%	50%	50%
4	32%	45%	45%	32%	45%	45%
6 or more	20%	40%	40%	8%	37%	37%





FULL-DEPTH FLEXIBLE  
 TRAFFIC FACTOR LESS THAN 15.0 (RURAL)  
 TRAFFIC FACTOR LESS THAN 10.0 (URBAN)  
 ROUTE- Elin O'Hare  
 SECTION- 03-96-0021  
 COUNTY- Dupage  
 LOCATION- at I-250 (EO Expy)

22-Aug-12  
 2:33 PM  
 FULL DEPTH FLEXIBLE PAVEMENT  
 MAINTENANCE COSTS

PROJECT LENGTH (FT) 5157  
 AVERAGE LANE WIDTH (FT) 19.73  
 NUMBER OF LANES 1  
 # OF EDGES 2  
 INSIDE SHOULDER WIDTH (FT) 8  
 OUTSIDE SHOULDER WIDTH (FT) 6  
 # OF CENTERLINES 1  
 PROJECT TYPE 2  
 PAVING WIDTH 1  
 INTERSTATE / OTHER ROUTE 1  
 FLEXIBLE THICKNESS- 11.5  
 TRAFFIC FACTORS MINIMUM 7.11 ACTUAL 7.76

22-Aug-12  
 2:33 PM

FULL DEPTH FLEXIBLE PAVEMENT  
 MAINTENANCE COSTS

Activity 1  
 YEAR 5 RAND/ITHERM CRACK ROUT & SEAL 50% (LF)  
 SHLDR JT ROUT & SEAL 100% (LF)  
 CENTERLINE JT ROUT & SEAL 100% (LF)  
 PARTIAL PVMIT PATCH 0.1% (SQ YDS)  
 \$19,297

Activity 2  
 YEAR 10 PARTIAL PVMIT PATCH 0.5% (SQ YDS)  
 RAND/ITHERM CRACK ROUT & SEAL 50% (LF)  
 SHLDR JT ROUT & SEAL 100% (LF)  
 CENTERLINE JT ROUT & SEAL 100% (LF)  
 \$23,437

Activity 3  
 YEAR 15 2" MILL PVMIT & SHLDR 100% (SQ YDS)  
 PARTIAL PVMIT PATCH 1.0% (SQ YDS)  
 2" OVERLAY PVMIT & SHLDR 100% (TONS)  
 \$33,822

Activity 4  
 YEAR 20 SHLDR JT ROUT & SEAL 100% (LF)  
 CENTERLINE JT ROUT & SEAL 100% (LF)  
 RAND/ITHERM CRACK ROUT & SEAL 50% (LF)  
 PARTIAL PVMIT PATCH 0.1% (SQ YDS)  
 \$172,794

Activity 5  
 YEAR 25 SHLDR JT ROUT & SEAL 100% (LF)  
 CENTERLINE JT ROUT & SEAL 100% (LF)  
 RAND/ITHERM CRACK ROUT & SEAL 50% (LF)  
 PARTIAL PVMIT PATCH 0.5% (SQ YDS)  
 \$10,685

Activity 6  
 YEAR 30 2" MILL PVMIT & SHLDR 100% (SQ YDS)  
 PARTIAL PVMIT PATCH 2.0% (SQ YDS)  
 HMA SHLDR PATCHING 1.0% (SQ YDS)  
 POLICY HMA OVERLAY PVMIT (TONS)  
 POLICY HMA OVERLAY SHLDR (TONS)  
 \$154,920

Activity 7  
 YEAR 35 SHLDR JT ROUT & SEAL 100% (LF)  
 CENTERLINE JT ROUT & SEAL 100% (LF)  
 RAND/ITHERM CRACK ROUT & SEAL 50% (LF)  
 PARTIAL PVMIT PATCH 0.1% (SQ YDS)  
 \$6,858

Activity 8  
 YEAR 40 SHLDR JT ROUT & SEAL 100% (LF)  
 CENTERLINE JT ROUT & SEAL 100% (LF)  
 RAND/ITHERM CRACK ROUT & SEAL 50% (LF)  
 PARTIAL PVMIT PATCH 0.5% (SQ YDS)  
 \$7,186

INITIAL COSTS

ITEM	QUANTITY	UNIT PRICE	COST
SURFACE (SQ YDS)	11,305	\$11.65	\$131,703
POLY BINDER (SQ YDS)	11,305	\$8.66	\$97,901
BINDER (SQ YDS)	11,305	\$39.03	\$441,234
SHOULDERS (SQ YDS)	8022	\$54.34	\$435,915
SUBBASE GRAN MATL TY C (TONS)	0		\$0
CONSTRUCTION INITIAL COST (PW)			\$1,106,753
TOTAL REHABILITATION COST (PW)			\$397,722
TOTAL LIFE CYCLE COST (PW)			\$1,504,475
ANNUAL COST PER MILE			\$62,620

UNIT COST

ITEM	QUANTITY	UNIT PRICE	COST
RAND/ITHERM CRACK ROUT & SEAL (LF)	11,305	\$1.75	\$19,784
SHLDR JT ROUT & SEAL (LF)	226	\$90.00	\$20,340
CENTERLINE JT ROUT & SEAL (LF)	80	\$90.00	\$7,200
2" MILL PVMIT & SHLDR (SQ YDS)	2,374	\$104.02	\$246,938
2" OVERLAY PVMIT & SHLDR (TONS)	786	\$104.02	\$81,758
2" MILL PVMIT ONLY (SQ YDS)	11	\$90.00	\$990
HMA SHOULDER PATCHING (SQ YDS)			
POLICY HMA OVERLAY PVMIT (TONS)			
POLICY HMA OVERLAY SHLDR (TONS)			
UNIT COST			\$376,020
TOTAL REHABILITATION COST (PW)			\$1,106,753
TOTAL REHABILITATION COST (PW)			\$397,722
TOTAL LIFE CYCLE COST (PW)			\$1,504,475
ANNUAL COST PER MILE			\$62,620

MAINTENANCE COSTS:

ITEM	QUANTITY	UNIT PRICE	COST
RAND/ITHERM CRACK ROUT & SEAL (LF)	11,305	\$1.75	\$19,784
SHLDR JT ROUT & SEAL (LF)	226	\$90.00	\$20,340
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2" MILL PVMIT ONLY (SQ YDS)	11	\$90.00	\$990
HMA SHOULDER PATCHING (SQ YDS)			
POLICY HMA OVERLAY PVMIT (TONS)			
POLICY HMA OVERLAY SHLDR (TONS)			
UNIT COST			\$376,020
TOTAL REHABILITATION COST (PW)			\$1,106,753
TOTAL REHABILITATION COST (PW)			\$397,722
TOTAL LIFE CYCLE COST (PW)			\$1,504,475
ANNUAL COST PER MILE			\$62,620

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UNIT COST			\$376,020
TOTAL REHABILITATION COST (PW)			\$1,106,753
TOTAL REHABILITATION COST (PW)			\$397,722
TOTAL LIFE CYCLE COST (PW)			\$1,504,475
ANNUAL COST PER MILE			\$62,620

MATERIAL TYPE/PERCENTAGE PCC 19.0%

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Total Rehabilitation Cost (Present Worth)

\$397,722